# Avila Cottages

Draft Transportation Impact Study

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### **Executive Summary**

This study evaluates the potential transportation impacts of the proposed Avila Cottages project located east of Wild Cherry Canyon and north of Avila Beach Drive in unincorporated San Luis Obispo County. The project would develop 50 rental cottages, support facilities including a 60 seat restaurant and 35-40 seat bar, a small spa and fitness area and would provide public parking consistent with the San Luis Bay Coastal Area Plan to be used by guests, visitors and employees. The weekday project trip generation estimate shows 842 new daily trips and 63 new PM peak hour trips. The Saturday estimate shows 949 new daily trips and 84 new MD peak hour trips.

Ten intersections were evaluated during the weekday afternoon (4-6 PM) and summer Saturday midday (11 AM-1 PM) time period, and five roadway segments were evaluated using ADT thresholds for County operated facilities and during the PM peak hour for Caltrans facilities.

The following deficiencies and recommended improvements are noted under Existing Plus Project conditions:

- Avila Beach Drive/Ontario Road (#9): The addition of project traffic degrades conditions on the southbound approach, which operates at LOS D during the weekday PM peak hour and LOS E during the Saturday MD peak hour under Existing Plus Project conditions. This intersection meets the peak hour signal warrant. Signalization of this intersection would result in acceptable operations and is included in the County's Roadway Impact Fee program
- Avila Beach Drive/US 101 SB Ramps/Shell Beach Road (#10): The addition of project traffic worsens delay on the side street approaches, with the worst approach continuing to operate at LOS D during the weekday PM peak hour and degrading to LOS F during the Saturday MD peak hour under Existing Plus Project conditions. This intersection meets the peak hour signal warrant. Caltrans approved a Project Study Report-Project Development Support document in May 2016 and the County is proceeding with the Project Approval and Environmental Document phase for interchange improvements at this location. The Intersection Control Evaluation for this intersection recommends a single lane roundabout, which would result in acceptable operations. These improvements are included in the County's Roadway Impact Fee program.

Cumulative conditions reflect buildout of land uses in the area and include roadway improvements funded in the County's Roadway Impact Fee program. No improvements are recommended to address Cumulative deficiencies.

The following recommendations are provided to improve site access and on-site circulation:

- Improve the Project Driveway/Ana Bay Road intersection to provide acceptable sight distance by trimming vegetation that obstructs the sight distance.
- Develop a pedestrian path of travel to Avila Beach Drive.

The analysis supporting these recommendations is provided in the body of this report.

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## Introduction

This study evaluates the potential transportation impacts of the proposed Avila Cottages project located on Ana Bay Road east of Wild Cherry Canyon and north of Avila Beach Drive in unincorporated San Luis Obispo County. The project includes the development of 50 rental cottages and support facilities including a 50 seat restaurant and 35-40 seat bar and public parking as required by the San Luis Bay Coastal Area Plan (LCP). The project's components are described in detail in Table 6.

The project's location and study intersections are shown on **Figure 1**, and **Figure 2** shows the site plan. The following intersections were evaluated during the weekday afternoon (4-6 PM) and Saturday mid-day (11 AM-1 PM) time periods:

- 1. Project Driveway/Ana Bay Road
- 2. Avila Beach Drive/Ana Bay Road
- 3. Avila Beach Drive/First Street
- 4. Avila Beach Drive/San Miguel Street
- 5. Avila Beach Drive/San Luis Street
- 6. Avila Beach Drive/San Luis Bay Drive
- 7. San Luis Bay Drive/Ontario Road
- 8. San Luis Bay Drive/US 101 NB Ramps
- 9. Avila Beach Drive/Ontario Road
- 10. Avila Beach Drive/US 101 SB Ramps/Shell Beach Road

The following roadway segments were evaluated using ADT thresholds for County operated facilities and during the PM peak hour for Caltrans facilities:

- Avila Beach Drive (West of San Luis Bay Drive)
- Avila Beach Drive (West of US 101)
- San Luis Bay Drive (West of US 101)
- US 101 (North of San Luis Bay Drive)
- US 101 (South of Avila Beach Drive)

The study intersections were evaluated under these analysis scenarios:

- 1. **Existing Conditions** reflect recently collected traffic counts and the existing transportation network.
- 2. Existing Plus Project Conditions add project generated traffic to Existing Conditions volumes.
- 3. **Cumulative Conditions** reflect future traffic conditions reflective of buildout of the Avila Beach area, developed using the Avila Traffic Model.
- 4. Cumulative Plus Project Conditions add project traffic to Cumulative Conditions volumes.

A description of the analysis approach follows Figures 1 and 2.

Figure 1: Project and Study Locations





Legend: - Project Site (7) - Study Intersection

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# Figure 2: Site Plan



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# **Analysis Methods and Thresholds**

The analysis approach was developed based on the County of San Luis Obispo and Caltrans standards.

#### **County Facilities**

The County's Traffic Impact Study policies provide guidelines for identifying transportation impacts. The study intersections are located within the Avila Beach Urban Reserve Line, where Level of Service (LOS) D is acceptable but LOS E is not. The San Luis Bay Coastal Area Plan (Chapter 8, Section B2) provides further guidance that Avila Beach Drive shall not be subject to traffic levels exceeding LOS C based on counts conducted on a weekday in May.

In July 2016 the County Board of Supervisors directed staff to require all Avila Beach projects subject to CEQA requiring a traffic study evaluate both the traffic impacts during the second week of May and during the most appropriate time period relative to the proposed land use as an interim approach until the Avila Beach Community Plan Update is finalized. The summer Saturday mid-day time period was identified as the most appropriate time period relative to the proposed hotel in consultation with County staff.

For side-street-stop controlled intersections, an impact is identified if the project results in LOS E or worse operations (LOS D or worse on Avila Beach Drive) on the stop-controlled approach and the project meets the peak hour signal warrant. Per the CA-MUTCD, *"The satisfaction of a traffic signal warrant or warrants shall not in itself require installation of a traffic control signal."* The satisfaction of the peak hour signal warrant should therefore be considered as justification to conduct a more detailed warrant study evaluating the need for a new traffic signal.

#### **Caltrans Facilities**

Caltrans controls the US Highway 101 mainline and ramps and relies on LOS to identify impacts. Caltrans strives to maintain operations at the LOS C/D threshold on state-operated facilities, where LOS C is acceptable but LOS D is not. If an existing State Highway facility is operating at LOS D, E, or F the existing service level should be maintained.

#### Level of Service Thresholds

The level of service thresholds for intersections based on the 2010 Highway Capacity Manual (HCM) are presented in Table 1. The study intersections were analyzed with the Synchro 9 software package applying the 2010 HCM methods. For side-street-stop controlled intersections the overall intersection average delay per vehicle is provided followed by the worst approach average delay per vehicle in parenthesis. The LOS is reported only for the worst approach for these intersections consistent with the HCM.

Table 1: Intersection Level of Service Thresholds						
Signalized Intersecti	ons <sup>1</sup>	Stop Sign Controlled Intersections <sup>2</sup>				
Control Delay (seconds/vehicle)	Level of Service	Control Delay (seconds/vehicle)	Level of Service			
$\leq 10$	А	$\leq 10$	А			
> 10 - 20	В	> 10 - 15	В			
> 20 - 35	С	> 15 - 25	С			
> 35 - 55	D	> 25 - 35	D			
> 55 - 80	Е	> 35 - 50	Е			
> 80	F	> 50	F			
1. Source: Exhibit 18-4 of the 2010 Highnay Capacity Manual.						
2. Source: Exhibits 19-1 and 20-2 of the 201	10 Highway Capacity Man	uual.				

Roadway segment thresholds are summarized in Table 2 and Table 3. The mainline segments of US Highway 101 were evaluated using a vehicle density calculation using the Highway Capacity Software consistent with the 2010 HCM.

Table 2: Freeway Segment Level of Service Thresholds				
Density (passenger car/mile/lane)	Level of Service			
$\leq 11$	А			
> 11 - 18	В			
> 18 - 26	С			
> 26 - 35	D			
> 35 - 45	Е			
> 45 (demand exceeds capacity)	F			
1. Source: Table 1 of the 2010 Highway Capacity I	Manual.			

The study roadway segments Avila Beach Drive and San Luis Bay Drive were evaluated using generalized daily volume thresholds based on the LCP.

Table 3: Arterial Segments Level of Service Thresholds							
Two-Lane Undivided Arterial Segment (With LTL) <sup>1</sup> Two-Lane Undivided Arterial Segment (No LTL) <sup>1</sup>							
Average Daily Two-Way Volume	Level of Service	Average Daily Two-Way Volume	Level of Service				
≤ 11,000	А	≤ 9,000	А				
> 11,000 - 12,500	В	> 9,000 - 10,500	В				
> 12,500 - 14,500	С	> 10,500 - 12,000	С				
> 14,500 - 16,000	D	> 12,000 - 13,500	D				
> 16,000 - 18,000	Е	> 13,500 - 15,000	Е				
> 18,000	F	> 15,000	F				
1. Source: 2015 Avila Valley Circulation Stud	ly Update, which estima	ated capacities by using HCM 2000 methodo	ologies.				

# **Existing Conditions**

This section describes the existing transportation system and current operating conditions in the study area.

#### **EXISTING ROADWAY NETWORK**

US Highway 101 is a north-south facility connecting Los Angeles to San Francisco. In the vicinity of the project it is a four-lane freeway with full access interchanges at Avila Beach Drive and San Luis Bay Drive. There is an auxiliary lane serving southbound traffic roughly between the San Luis Bay Drive and Spyglass Drive interchanges.

Avila Beach Drive is an east-west arterial connecting US Highway 101 with Port San Luis. Along the project frontage it consists of two lanes and is posted with a speed limit of 40 miles per hour (mph).

San Luis Bay Drive is an arterial roadway connecting US Highway 101 to Avila Beach Drive, generally used for local trips and trips to and from the north on US Highway 101.

*First Street* is a collector roadway serving the downtown area of Avila Beach. The signalized intersection of First Street and Avila Beach Drive serves high volumes of pedestrians and cyclists and provides a controlled crossing location with a dedicated pedestrian and bicycle phase.

Ana Bay Road is a local road serving the project site and the Avila Beach Golf Resort. It is stop controlled where it intersects with Avila Beach Drive.

#### EXISTING PEDESTRIAN AND BICYCLE FACILITIES

Pedestrian facilities include sidewalks, crosswalks, multi-use paths, and pedestrian signals at signalized intersections. The First Street/Avila Beach Drive intersection provides pedestrian crosswalks and a dedicated pedestrian/bicycle phase.

Bicycle facilities consist of multi-use paths separate from the roadway (Class I), on-street striped bike lanes (Class II), and signed bike routes (Class III). The Bob Jones City-to-the-Sea Trail is mostly a Class I multi-use path within Avila Beach. Avila Beach Drive is a Class III route east of San Luis Bay Drive, and is identified as a recreational route west of San Luis Bay Drive. San Luis Bay Drive is a Class III bike route.

#### **EXISTING TRANSIT SERVICE**

The Avila Beach Trolley provides free fixed route service between Pismo Beach and Avila Beach on Friday evenings, Saturdays, and Sundays during the summer. Weekend service is provided hourly between 10 AM and 6 PM. The Trolley connects to the South County Transit network which serves the Five Cities area with four routes.

The San Luis Obispo Regional Transit Authority (RTA) provides regional fixed-route and dial-a-ride services to San Luis Obispo County. Route 10 serves the South County, with a stop in Pismo Beach.

#### **EXISTING TRAFFIC CONDITIONS**

Traffic counts for weekday PM peak hour conditions were collected at study intersections 3-10 in September 2014 as a part of the Avila Circulation Study. Intersections 1 and 2 were collected in June 2016, with volumes on Avila Beach drive adjusted to match the volumes at Avila Beach Drive/First Street when counted as a part of the Avila Circulation Study.

In July 2016 the County Board of Supervisors directed staff to require all Avila Beach projects subject to CEQA requiring a traffic study evaluate both the traffic impacts during the second week of May and during the most appropriate time period relative to the proposed land use as an interim approach until the Avila Beach Community Plan Update is finalized. The summer Saturday mid-day time period was identified as the peak travel period for both Avila Beach and typical hotels in consultation with County staff.

Traffic counts for summer Saturday mid-day peak hour conditions were collected at all study intersections on August 20, 2016. The traffic volumes on this day were compared to other Saturdays in June, July, and August 2016 (including holiday weekend Saturdays) using data from the County's permanent count station on Avila Beach Drive. The Saturday counts were found to be 7% below the average summer Saturday volumes. Accordingly, the Saturday counts at all study intersections were increased by 7% to represent the average summer Saturday.

Traffic counts on US Highway 101 were obtained from SLOCOG's Highway 101 Mobility Study data. The traffic count sheets are included in Appendix A.

**Figure 3** shows the existing peak hour traffic volumes and lane configurations. Table 4 shows the existing peak hour delay and corresponding LOS based on the thresholds shown in Table 1. Detailed calculation sheets are provided in Appendix B.

Table 4: Existing Intersection Peak Hour LOS						
Intersection	Peak Hour	Delay $(sec/veh)^1$	LOS <sup>2</sup>			
1 Droiget Drivoway / Ang Bay Road	Weekday PM	0.0 (0.0)	- (A)			
1. Project Driveway/ Ana Day Koad	Saturday MD	0.0 (0.0)	- (A)			
2 Avila Roach Drive / Ana Ray Road	Weekday PM	1.1 (18.3)	- (C)			
2. Aviia Deach Dhve/ Alla Day Road	Saturday MD	1.0 (13.7)	- (B)			
2 Avila Reach Drive / First Street	Weekday PM	19.1	В			
5. Aviia Deach Drive/ Prist Street	Saturday MD	21.8	С			
4 Avila Reach Drive/San Migual Street	Weekday PM	1.5 (19.6)	- (C)			
4. Aviia Deach Drive/ San Miguer Street	Saturday MD	1.6 (14.2)	- (B)			
5 Avila Beach Drive/San Luis Street	Weekday PM	2.1 (21.7)	- (C)			
5. Avila Deach Drive/ San Luis Street	Saturday MD	1.6 (14.7)	- (B)			
( Arile Beach Drive/San Luis Bay Drive	Weekday PM	8.8	А			
0. Aviia Deach Drive/ San Luis Day Drive	Saturday MD	14.7	В			
7 San Luis Ray Drive (Ontario Road	Weekday PM	3.4 (25.6)	- (D)			
7. San Luis Day Drive/ Ontario Road	Saturday MD	3.1 (10.7)	- (B)			
e San Luis Ray Drive /US 101 NR Pampa	Weekday PM	9.0 (35.5)	- (E)			
8. San Luis Day Drive/ 05 101 ND Kamps	Saturday MD	6.6 (14.9)	- (B)			
0 Arrile Reach Drive (Ontario Read	Weekday PM	2.7 (23.9)	- (C)			
9. Avila Beach Drive/ Ontario Road	Saturday MD	4.9 (32.6)	- (D)			
10. Avila Beach Drive/US 101 SB Ramps/Shell	Weekday PM	4.5 (31.7)	- (D)			
Beach Road	Saturday MD	9.1 (47.7)	- (E)			
1. HCM 2010 average control delay in seconds per vel	nicle (HCM 2000 u	used for Intersection 3).				
2. For side-street-stop controlled intersections the wor	rst approach's dela	y is reported in parenthes	is.			
Unacceptable operations shown in <i>bold italics</i> .						

The overall intersection LOS for side-street-stop controlled intersections are not reported, consistent with the HCM. Intersections 8, 9, and 10 have approaches with the worst delay below the desired LOS.

During field observations, eastbound queues at the Avila Beach Drive/First Street intersection extended the length of the bridge during the weekday PM peak and reached the bridge during the Saturday mid-day peak.

Table 5 summarizes the existing arterial segment LOS and existing freeway segment LOS based on the thresholds shown in Tables 2 and 3.

Table 5: Existing Weekday Segment Operations					
Arterial Segment		Two-way Daily Volume	LOS		
Avila Beach Drive (West of San Luis F	Bay Drive)	11,136	$B^1$		
Avila Beach Drive (West of US 101)		9,631	$A^1$		
San Luis Bay Drive (West of US 101)		8,010	$A^2$		
E C A		PM Peak Hour Density	LOC		
Freeway Segment		(pc/mi/ln) <sup>3</sup>	LUS		
US Highway 101 north of San Luis Ba	y Drive				
	Northbound	23.3	С		
	Southbound	28.6	D		
US Highway 101 south of Avila Beach	Drive				
	Northbound	17.2	В		
	Southbound	30.9	D		
1. Two-Lane Undivided Arterial Segment	(With LTL). Source	e: Avila Criculation Study Update, 2015.			
2. Two-Lane Undivided Arterial Segment	(No LTL). Source:	Avila Circulation Study Update, 2015			
3. Passenger cars per mile per lane					
Unacceptable operations shown in <b>bold in</b>	talics.				

Both study segments of southbound US Highway 101 operate at LOS D during the weekday PM peak hour, which is below Caltrans' desired LOS C service level. All other study segments operate at LOS C or better.

Figure 3: Existing Peak Hour Volumes and Lane Configuration



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# **Existing Plus Project Conditions**

This section evaluates the impacts of the proposed project on the surrounding transportation network. Existing Plus Project conditions reflect existing traffic levels plus the estimated traffic generated by the proposed project.

#### **PROJECT TRAFFIC ESTIMATES**

The amount of project traffic affecting the study intersections is estimated in three steps: trip generation, trip distribution, and trip assignment. Trip generation refers to the total number of new trips generated by the site. Trip distribution identifies the general origins and destination of these trips, and trip assignment specifies the routes taken to reach these origins and destinations.

#### Trip Generation

The project's trip generation estimates, summarized in Table 6, were developed using rates in the Institute of Transportation Engineers' (ITE) *Trip Generation Manual*.

Table 6: Project Trip Generation									
			Week	day PN	1 Peak		Satur	day MI	) Peak
ITE Land Use		Weekday		Hour		Saturday		Hour <sup>4</sup>	
(Code)	Size	Daily	In	Out	Total	Daily	In	Out	Total
Sit Down	4,600 s.f.	585	27	18	45	729	34	31	65
$\mathbf{P}_{\text{ostaurant}} \left( 032 \right)^{1}$	Diverted Link Trips <sup>2</sup>	-152	-7	-5	-12	-190	-9	-8	-17
Restaurant (752)	Restaurant Net New Trips	433	20	13	33	539	25	23	48
Hotel $(310)^3$	50 units	409	15	15	30	410	20	16	36
	Total Driveway Trips	994	42	33	75	1,139	54	47	101
	Net New Trips	842	35	28	63	949	45	39	84

1. ITE Trip Generation Manual, Land Use Code 932, Sit-Down Restaurant. Average rate used for Daily and Peak Hour trips.

2. Diverted Link Trip reductions based on average of data in ITE manual (26% for restaurant).

3. ITE Trip Generation Manual, Land Use Code 310, Hotel. Average rate used for Daily and Peak Hour trips.

4. Saturday Mid-Day Peak Hour not available in ITE Trip Generation Manual, data used is Saturday Peak Hour of Generator.

Source: ITE Trip Generation Manual, 9th Edition, 2012; CCTC, 2016.

The project trips are categorized as new trips and diverted link trips. Diverted link trips are attracted from the existing traffic volume on roadways near the project site but without direct access to the site. It requires a diversion from a roadway not adjacent to the site to another roadway to gain direct access to the site. A diverted link trip adds traffic to Ana Bay Road and shifts a trip on Avila Beach Drive. New trips are from drivers whose primary destination is located on the project site. These new trips increase traffic on the study roadway segments and at the study intersections. Diverted link trip reductions were applied to the restaurant component of the project, since typically a portion of traffic from this use are trips already on the adjacent roadway network without direct access to the site.

This is a conservative estimate of the project's traffic generating uses, and typical operations are expected to generate fewer trips. Most hotels surveyed by ITE included supporting uses such as restaurants and cocktail lounges. The estimate in Table 6 considers restaurant trips and hotel trips to be independent, potentially overstating the number of trips generated.

The weekday project trip generation estimate shows 842 new daily trips and 63 new PM peak hour trips. The Saturday estimate shows 949 new daily trips and 84 new MD peak hour trips.

#### Trip Distribution and Assignment

The directions of approach and departure for project trips were estimated based on existing trip patterns and the locations of complementary land uses. Project trips were assigned to individual intersections based on the trip distribution percentages, and were then added to the existing traffic volumes for Existing Plus Project Conditions. **Figure 4** shows the trip distribution and project trip assignment.

#### IMPACT ANALYSIS

Table 7 summarizes the automobile operating conditions at intersections under Existing and Existing Plus Project conditions. Detailed LOS calculation sheets are provided in Appendix B and signal warrant calculation sheets are provided in Appendix D. **Figure 5** shows the Existing Plus Project peak hour traffic volumes.

Table 7: Existing and Existing Plus Project Intersection Peak Hour LOS							
		Existing Existing Plus Project				Project	
		Delay		Delay		Warrant	
Intersection	Peak Hour	(sec/veh) <sup>1</sup>	LOS <sup>2</sup>	(sec/veh) <sup>1</sup>	LOS <sup>2</sup>	Met? <sup>3</sup>	
1 Project Drivoway / Area Bay Pood	Weekday PM	0.0 (0.0)	- (A)	5.3 (8.5)	- (A)	-	
1. Project Driveway/Ana Bay Koad	Saturday MD	0.0 (0.0)	- (A)	5.2 (8.6)	- (A)	-	
2 Avila Boach Drive / Ana Bay Poad	Weekday PM	1.1 (18.3)	- (C)	1.9 (21.1)	- (C)	-	
2. Aviia Beach Drive/ Alia Bay Road	Saturday MD	1.0 (13.7)	- (B)	2.0 (15.1)	- (C)	-	
2 Arrile Reade Drives / First Streat	Weekday PM	19.1	В	22.9	С	-	
5. Aviia Beach Drive/ First Street	Saturday MD	21.8	С	22.2	С	-	
4 Arrile Reach Drive /San Microel Street	Weekday PM	1.5 (19.6)	- (C)	1.5 (20.5)	- (C)	-	
4. Avna Beach Drive/ San Miguel Street	Saturday MD	1.6 (14.2)	- (B)	1.6 (15.4)	- (C)	-	
5 Avrila Roach Drive /San Luis Streat	Weekday PM	2.1 (21.7)	- (C)	2.1 (22.5)	- (C)	-	
5. Avna Beach Drive/ San Luis Street	Saturday MD	1.6 (14.7)	- (B)	1.6 (14.7)	- (B)	-	
( Arrile Reach Drive /San Luis Per Drive	Weekday PM	8.8	А	9.2	А	-	
0. Avita Beach Drive/ San Luis Day Drive	Saturday MD	14.7	В	15.4	В	-	
7 San Luis Pau Drive (Ontario Road	Weekday PM	3.4 (25.6)	- (D)	3.5 (26.9)	- (D)	-	
7. San Luis Bay Drive/Ontario Road	Saturday MD	3.1 (10.7)	- (B)	2.9 (10.9)	- (B)	-	
8 San Luis Pau Drive /US 101 NP Parana	Weekday PM	9.0 (35.5)	- (E)	9.1 (37.4)	- (E)	No	
6. San Luis Bay Drive/ 05 101 NB Kamps	Saturday MD	6.6 (14.9)	- (B)	6.8 (15.6)	- (C)	No	
0 Arrile Reach Drive (Ontario Read	Weekday PM	2.7 (23.9)	- (C)	2.8 (25.1)	- (D)	Yes	
9. Aviia Beach Drive/Ontario Road	Saturday MD	4.9 (32.6)	- (D)	5.2 (35.8)	- (E)	Yes	
10. Avila Beach Drive/US 101 SB Ramps/Shell	Weekday PM	4.5 (31.7)	- (D)	4.6 (34.0)	- (D)	Yes	
Beach Road	Saturday MD	9.1 (47.7)	- (E)	10.2 (58.1)	- (F)	Yes	
1. HCM 2010 average control delay in seconds per ve	hicle (HCM 2000	used for Inters	ection 3)				
2 Energial stand the second stand interventions the second			.1	· ·			

2. For side-street-stop controlled intersections the worst approach's delay is reported in parenthesis.

3. Based on California MUTCD Warrant 3

Unacceptable operations shown in **bold italics**.

The following deficiencies and recommendations are noted:

- San Luis Bay Drive/US 101 NB Ramps (#8): The addition of project traffic worsens delay on the northbound off-ramp approach, which continues to operate at LOS E during the weekday PM peak hour under Existing Plus Project conditions. This intersection does not meet the peak hour signal warrant, so no improvements are recommended.
- Avila Beach Drive/Ontario Road (#9): The addition of project traffic degrades conditions on the southbound approach, which operates at LOS D during the weekday PM peak hour and LOS E during the Saturday MD peak hour under Existing Plus Project conditions. This

intersection meets the peak hour signal warrant. Signalization of this intersection would result in acceptable operations and is included in the County's Roadway Impact Fee program

Avila Beach Drive/US 101 SB Ramps/Shell Beach Road (#10): The addition of project traffic worsens delay on the side street approaches, with the worst approach continuing to operate at LOS D during the weekday PM peak hour and degrading to LOS F during the Saturday MD peak hour under Existing Plus Project conditions. This intersection meets the peak hour signal warrant. Caltrans approved a Project Study Report-Project Development Support document in May 2016 and is proceeding with the Project Approval and Environmental Document phase for interchange improvements at this location. The Intersection Control Evaluation for this intersection recommends a single lane roundabout, which would result in acceptable operations. These improvements are included in the County's Roadway Impact Fee program.

All other study intersections are expected to operate acceptably with the addition of project traffic.

		Existing	PM	Existing Plus I	Existing Plus Project PM	
		Two-way		Two-way		
Arterial Segment		Volume	LOS	Volume	LOS	
Avila Beach Drive (West of San Luis Bay	Drive)	11,136	В	11,810	В	
Avila Beach Drive (West of US 101)		9,631	А	9,968	А	
San Luis Bay Drive (West of US 101)		8,010	А	8,347	А	
Freeway Segment		Density (pc/mi/ln) <sup>1</sup>	LOS	Density (pc/mi/ln) <sup>1</sup>	LOS	
US Highway 101 north of San Luis Bay I	Drive					
	Northbound	23.3	С	23.5	С	
	Southbound	28.6	D	28.8	D	
	Soumbound					
US Highway 101 south of Avila Beach D	rive					
US Highway 101 south of Avila Beach D	Prive Northbound	17.2	В	17.3	В	

Table 8 summarizes the segment operations under Existing and Existing Plus Project Conditions.

The addition of project traffic would not change the service level for any of the study segments. The southbound segments of US Highway 101 would continue to operate unacceptably. The addition of project traffic increases vehicle density by less than one percent.

#### **Bicycles**

Bicycle deficiencies would occur if the project disrupts existing or planned bicycle facilities or is otherwise incongruent with the County's Bikeways Plan. There is a multi-use path planned parallel to Avila Beach Drive along the project frontage. The project would not conflict with this or any other planned bicycle facility. There are currently no bicycle facilities along or parallel to Ana Bay Road or along the project driveway.

#### **Pedestrians**

Pedestrian deficiencies would occur if the project fails to provide safe and accessible pedestrian connections between the project and nearby destinations.

There are currently no pedestrian facilities along the Project Driveway or Ana Bay Road. The walking distance from the project site to the Avila Beach Drive/First Street intersection is roughly 2/3 of a mile, and hotel guests could walk to other destinations like the Golf Club and coastal access points.

The project proposes electric shuttles for guests traveling between the hotel and destinations in Avila Beach. This will reduce pedestrian trips from the site to nearby destinations.

Pedestrian volumes crossing Avila Beach Drive should be monitored to determine the need, if any, of enhanced crossing treatments such Rectangular Rapid Flashing Beacons. Detailed site designs should be reviewed once they are available to ensure that pedestrian facilities are continuous and connect with likely destinations to the maximum extent possible.

#### Transit

Transit deficiencies would occur if the project disrupts existing or planned transit facilities or services or conflicts with County plans, guidelines, policies, or standards.

The nearest transit stop to the project is served by the seasonal Avila Trolley, which stops in front of San Luis Bay Inn, adjacent property to the Project Site, for northbound trips and at Avila Beach Drive at First Street when it is headed southbound, approximately ½ miles from the Project Driveway on Ana Bay Road. The project would not result in any transit deficiencies.

#### Turn Lane Evaluation

The need for left turn lanes on Avila Beach Drive at Ana Bay Road was evaluated based on the procedure described in the AASHTO Green Book (section 9.7.3), with Table 9-23 shown at right.

The eastbound speed limit is 40 miles per hour. During the existing PM peak hour, the opposing westbound traffic on Avila Beach Drive is 171 vehicles per hour (vph). The eastbound advancing volume consists of 767 vph existing trips with an additional 7 vph turning left. This translates to one percent of the advancing volume making left turns. Table 9-23 does not provide a threshold for less than five percent left turns. If the left turning volume reached five percent of the advancing volumes, the left turn would be warranted.

Currently fewer than ten vehicles make the eastbound left turning movement, and fewer than ten vehicles are projected to be added to this movement by the project during the peak hour. Installation of a left turn lane would likely conflict with the planned multi-use path on the south side of Avila Beach Drive due to limited right-of-way and topographic constraints. A left turn lane is not recommended due to the low turning volumes and these site-specific constraints.

	Site Access	and	<b>On-Site</b>	Circulation
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370

275

505

On-site circulation deficiencies would occur if project

designs fail to meet appropriate standards, fail to provide adequate truck access, or would result in hazardous or unsafe conditions.

100

240

The proposed site plan is shown on **Figure 2**. Primary project access will be provided via Ana Bay Road. The project access point currently exists. Emergency vehicle access is proposed via Blue Heron Drive, which would allow emergency vehicle access to San Luis Bay Drive without relying on Avila Beach Drive. Emergency access is also proposed via Wild Cherry Canyon Road.

No turn lanes are provided on Ana Bay Road at the project access point.

Stopping sight distance is the minimum length of clear roadway that must be visible for a motorist to stop for an object in the road. The stopping sight distance is sometimes used instead of intersection sight distance where restrictive conditions (such as excessive costs or immitigable environmental impacts) exist. For a design speed of 25 mph, the minimum stopping sight distance is 150 feet per the Caltrans Highway Design Manual.

Sight distance at the intersection of the project driveway and Ana Bay Road is constrained due to vegetation growth on the southwest corner. The sight line for a northbound driver is shown in the photo below, with the orange cone representing the location of a driver in a vehicle exiting the project driveway.



While vehicles exiting the driveway have an ample sight line looking north, the sight distance is only about 120 feet looking south. The vegetation on the southwest corner should be trimmed to ensure vehicles traveling northbound have adequate stopping sight distance approaching the intersection.

The turning radius for vehicles entering and exiting the project driveway is highly constrained. Turning templates should be used and the driveway should be reconstructed as necessary to ensure the design vehicle can turn in and out without encroaching on opposing traffic. Additionally, the centerline on Ana Bay Road should be restriped as a no-passing zone wherever adequate passing sight distance is not available.

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#### Parking

Chapter 22.18 of the County's Land Use Ordinance specifies parking requirements for the project. Table 9 summarizes the code-required parking.

Table 9: Code Required Parking Summary <sup>1</sup>						
Use		Rate	Size	Spaces		
	Customer Spaces	1 space per 60 sf of customer area plus	4,499	75.0		
Destaurant	Employee Spaces	1 space per 360 sf of customer area plus	4,499	12.5		
Restaurant	Employee Spaces	1 space per 100 sf of kitchen (active food prep)	2,084	20.8		
		Restauran	t Subtotal	108.3		
		2 spaces, plus		2		
Hotel		1 per unit, plus	50	50		
Hotel		1 per 10 units		5		
		Hote	l Subtotal	57		
		Total Code-Required	1 Spaces	166		
1. Source: County o	f SLO Parking Standar	ds defined in Chapter 22.18 of the Land Use Ordin	ance.			

Table 9 shows 166 parking spaces required for the site. No reductions have been applied for shared parking on site. The proposed parking supply is adequate.

Figure 4: Project Trip Distribution and Assignment



Figure 5: Existing Plus Project Volumes



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# **Cumulative Conditions**

Cumulative conditions represent build-out of the Avila Beach area.

#### CUMULATIVE ROADWAY NETWORK

The following network changes were assumed to be in place under Cumulative conditions:

- Avila Beach Drive/San Miguel Street (#4): Signal control and the addition of a 25-foot northbound right-turn pocket.
- Avila Beach Drive/San Luis Street (#5): Signal control and the addition of a 25-foot northbound right-turn pocket.
- San Luis Bay Drive/Ontario Road (#7): Signal control and the addition of 100-foot eastbound and westbound left-turn pockets.
- San Luis Bay Drive/US 101 NB Ramps (#8): Signal control with the existing lane configuration.
- Avila Beach Drive/Ontario Road (#9): Signal control with a 25-foot southbound right-turn pocket.
- Avila Beach Drive/US 101 SB Ramps/Shell Beach Road (#10): Single-lane roundabout with no bypass lanes. Per the Caltrans Roundabout Geometric Design Guidance, the California-specific values for critical headway and follow-up headway were used in the analysis.

The project would be required to contribute traffic impact fees which would apply towards future improvements in the area.

#### **CUMULATIVE TRAFFIC FORECASTS**

The Avila Traffic Model was developed to forecast future travel patterns in the Avila Beach area. The model incorporates future land uses to produce future year traffic forecasts. The most recent version of the model was calibrated and validated to year 2015 data and the future year scenario was developed to represent build-out conditions in the area, nominally for year 2035.

Cumulative forecasts for weekday PM conditions were obtained from the Avila Circulation Study. Cumulative forecasts for Saturday MD conditions were derived from the existing Saturday MD volumes using similar growth factors as was used for weekday PM.

#### **CUMULATIVE TRAFFIC CONDITIONS**

Table 10 summarizes traffic conditions at the study intersections under Cumulative and Cumulative Plus Project conditions. **Figure 6** and **Figure 7** depict Cumulative and Cumulative Plus Project condition traffic volumes, respectively.

Table 10: Cumulative and Cumulative Plus Project Intersection Peak Hour LOS								
		Cumulative Delay		Cumulative Plus Project				
				Delay		Warrant		
Intersection	Peak Hour	(sec/veh) <sup>1</sup>	LOS <sup>2</sup>	(sec/veh) <sup>1</sup>	LOS <sup>2</sup>	Met? <sup>3</sup>		
1. Project Driveway/Ana Bay Road	Weekday PM	0.0 (0.0)	- (A)	5.1 (8.5)	- (A)	-		
	Saturday MD	0.0 (0.0)	- (A)	4.7 (8.6)	- (A)	-		
2. Avila Beach Drive/Ana Bay Road	Weekday PM	1.3 (22.7)	- (C)	2.3 (27.8)	- (D)	No		
	Saturday MD	1.4 (15.3)	- (C)	2.4 (17.5)	- (C)	No		
3. Avila Beach Drive/First Street	Weekday PM	22.5	С	22.3	С	-		
	Saturday MD	19.7	В	20.2	С	-		
4. Avila Beach Drive/San Miguel Street	Weekday PM	5.5	А	5.6	А	-		
	Saturday MD	4.2	А	4.2	А	-		
5. Avila Beach Drive/San Luis Street	Weekday PM	7.3	А	7.7	А	-		
	Saturday MD	4.7	А	4.7	А	-		
6. Avila Beach Drive/San Luis Bay Drive	Weekday PM	12.9	В	13.5	В	-		
	Saturday MD	21.6	С	24.0	С	-		
7. San Luis Bay Drive/Ontario Road	Weekday PM	8.4	А	8.5	А	-		
	Saturday MD	5.0	А	5.0	А	-		
8. San Luis Bay Drive/US 101 NB Ramps	Weekday PM	7.9	А	8.1	А	-		
	Saturday MD	5.1	А	5.1	А	-		
9. Avila Beach Drive/Ontario Road	Weekday PM	5.6	А	5.6	А	-		
	Saturday MD	6.5	А	6.6	А	-		
10. Avila Beach Drive/US 101 SB	Weekday PM	10.8	В	11.0	В	-		
Ramps/Shell Beach Road	Saturday MD	8.6	А	8.9	А	-		
1. HCM 2010 average control delay in seconds per vehide (HCM 2000 used for Intersection 3).								
2. For side-street-stop controlled intersections the worst approach's delay is reported in parenthesis.								
3. Based on California MUTCD Warrant 3								

Unacceptable operations shown in *bold italics*.

The following intersection would operate below the desired LOS:

• Avila Beach Drive/Ana Bay Road (#2): The addition of project traffic degrades conditions on the southbound approach, which operates at LOS D during the weekday PM peak hour under Cumulative Plus Project conditions. This intersection does not meet the peak hour signal warrant, so no improvements are recommended.

All other study intersections are expected to operate acceptably with the addition of project traffic.

Table 11 summarizes the Cumulative traffic conditions along the study segments with and without the project.

Table 11: Cumulative and Cumulative Plus Project Weekday Peak Hour Segment LOS								
	Cumulative PM		<b>Cumulative Plus Project PM</b>					
	Two-way		Two-way	ŗ				
Arterial Segment	Volume	LOS	Volume	LOS				
Avila Beach Drive (West of San Luis Bay Drive)	12,936	С	13,610	С				
Avila Beach Drive (West of US 101)	11,131	В	11,468	В				
San Luis Bay Drive (West of US 101)	10,110	В	10,447	В				
	Density	1.00	Density	1.06				
Freeway Segment	(pc/mi/ln) <sup>1</sup>	LUS	(pc/mi/ln) <sup>1</sup>	LU8				
US Highway 101 north of San Luis Bay Drive								
Northbound	23.9	С	24.1	С				
Southbound	29.5	D	29.7	D				
US Highway 101 south of Avila Beach Drive								
Northbound	17.1	В	17.2	В				
Southbound	30.6	D	30.7	D				
1. Passenger cars per mile per lane.								
Daily Roadway Capacities LOS by Facility Type from Avila Circulation Study Update 2015.								
Unaceptable operations shown in <i>bold italics</i> .								

The southbound segments of US Highway 101 would operate at LOS D both with and without the project. All other study segments will continue to operate at LOS C or better. The addition of project traffic would not change the service level on US Highway 101 or the study segments.

Figure 6: Cumulative Volumes



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Figure 7: Cumulative Plus Project Volumes



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