# **Authors**

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# Nacimiento Water Project Intake Facility

Paul R. Kneitz, P.E., John R. Hollenbeck, P.E., Clay Haynes, P.E.

# ABSTRACT

Black & Veatch Corporation is directing a project team that is designing a new \$185 million supplemental raw water supply from Lake Nacimiento for San Luis Obispo County. The project consists of a multi-port sloping intake facility and pump station, two intermediate pump stations, three storage tanks, control center, and approximately 45-miles of transmission pipeline ranging in diameter from 36-inches to 12-inches.

This paper discusses the planning and design of the intake facility, which is a 180-foot deep, 16- to 20-foot diameter vertical shaft connected to the lake via a single 48- to 72-inch diameter microtunneled intake tunnel with a lake tap. A surface-mounted sloping intake with seven ports will allow water to be drawn from various depths of the reservoir for optimal water quality control.

Also addressed are the intake alternatives considered during conceptual and preliminary design, and detailed construction planning with the use of geotechnical baseline report (GBR) in the construction contract documents.

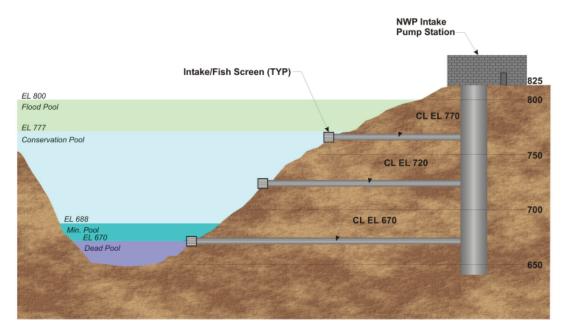
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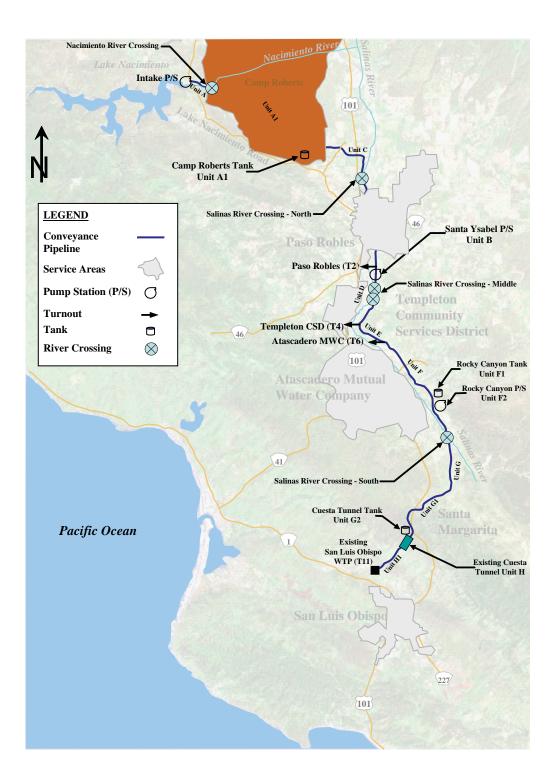
#### Introduction

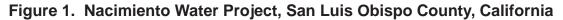
Located in the central coast of California, the San Luis Obispo County Flood Controll and Water Conservation District (District) is implementing the Nacimiento Water Project (NWP), a raw water conveyance system to deliver 15,750 acre-feet annually from Lake Nacimiento to participating agencies including City of Paso Robles, City of San Luis Obispo, Atascadero Mutual Water Company, Templeton Community Services District and County Service Area 10, Zone A (Figure 1). In 2005, Black & Veatch Corporation, Irvine, California, was selected to perform preliminary and final design of the \$185 million project, which is scheduled to be in operation by 2010. A key element of the Project is the NWP Intake, consisting of a 20-foot diameter, 180 foot deep, concrete-line vertical shaft connected to the Lake via a single 530-foot long, 48inch diameter micro tunnel with a lake tap. A surface-mounted sloping intake with seven ports will allow water to be drawn from various reservoir depths for optimal water quality. Construction is scheduled to begin winter of 2007.

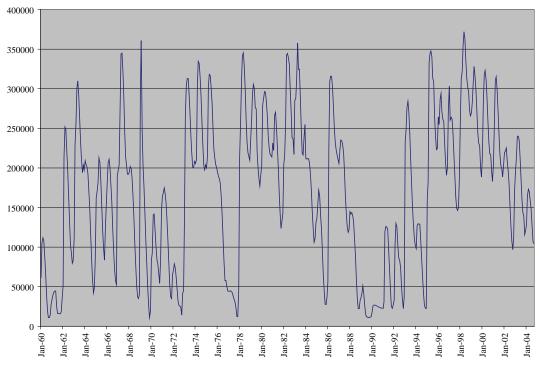
## Figure 2. NWP Intake Configuration in the EIR



Source: Black & Veatch Corporation, December 2005







Source: California Department of Water Resources Figure 3. Nacimiento Storage Levels, Acre-Feet (1960 to 2004)

#### NWP Intake Alternatives

The Nacimiento Water Project Final Environmental Impact Report (FEIR), December 2003, represented the conceptual design of the intake facility as a fixed three-port lake intake, with each port connected to the lake via a 72-inch diameter inlet tunnel (Figure 2). As design development progressed, it became clear that the fixed-port intake did not offer enough flexibility to withdraw raw water from varying lake levels (Figure 3) sufficient to optimize the water quality in combination with the participating agencies' treatment process. A water quality investigation and review of intake alternatives was subsequently conducted by Black & Veatch in December 2005.

A technical memorandum (TM) was prepared to summarize the existing lake water quality data and to provide recommendations on intake port depths, chemical fed options for the raw water supply, and water quality monitoring. Black & Veatch reviewed water quality data for Nacimiento Reservoir obtained from the District, as well as historical water levels, for the purpose of analyzing the following lake characteristics:

- Determining the position of the thermocline in the water column;
- Describing water quality in the epilimnion and hypolimnion and identifying locations (depths) where water quality changes occur; and
- Identifying additional water quality

data and monitoring needs.

Figure 3, obtained from the California Department of Water Resources website shows historical reservoir storage volumes based on acre-feet of storage, which was converted to reservoir elevations for analysis.

As expected, the water quality data indicated that during the winter months the temperature profiles in the reservoir are fairly uniform and stratification is not present. Beginning in February or March, the surface water start to warm and a thermocline starts to form. Usually during May, epilimnion, thermocline, and hypolimnion areas become distinct. The surface waters reach their highest temperatures in July and August and then start to cool. As the surface water cools, the thermocline erodes and the become temperature profiles again uniform. Stratification disappears as early as October or as late as December.

During the periods of a well established thermocline (May through September), the top of the thermocline was observed to be at depths of 15 to 30 feet and the bottom of the thermocline was at depths of 30 to 55 feet. The thermocline was usually 15 to 25 feet thick.

Based on the water quality data results, the number and spacing of intake ports were also reviewed as part of this TM. Ultimately, the decision was made to incorporate seven intake ports spaced at approximately 20 feet vertically in order to provide the flexibility to withdraw optimal water quality for any operating lake level. Figure 4 shows a schematic of the intake configuration accepted for final design.

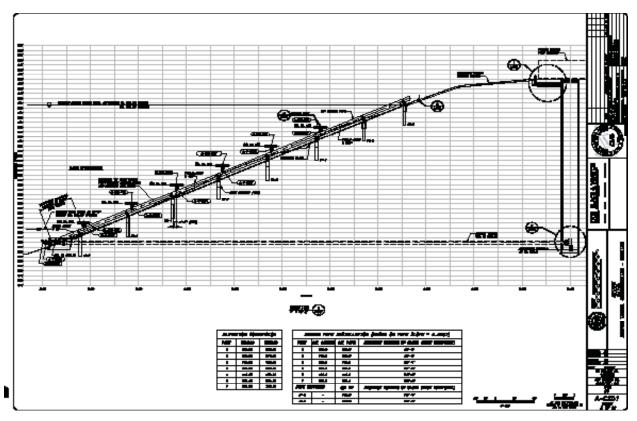
# NWP Intake Design

Final design of the NWP Intake focused on construction of the four principal features of the intake, namely, vertical shaft, intake tunnel, sloping intake and intake ports, and marine works. The following discusses the considerations involved with each part.

# Geologic & Geotechnical Considerations

The intake site is located in the central California Coast Ranges Geomorphic Province, characterized by moderately rugged terrain and north-northwest trending ridges and intervening alluvial valleys. The site is located in a tectonically and seismically active region dominated by the San Andreas Fault System.

As outline in the Geotechnical Baseline Report (GBR) for the intake, the intake shaft is located upstream of the existing Nacimiento Dam and adjacent to the north abutment and spillway. Geologic mapping indicates shaft and tunnel construction will take place in the Vagueros Formation consisting of moderately lithified, massive, poorly- to well-graded sandstone. The formation is predominantly quartz with minor constituents that included feldspar and clay minerals. The formation can include conglomerates and granitic boulders, as well as thing partings of clay, claystone, or siltstone that separate the massive



Source: Black & Veatch Corporation, 2007 Figure 4. NWP Intake Configuration Accepted for Final Design

sandstone beds.

The Vaqueros Formation observed at the boring locations varies significantly in strength, hardness, and quality. In general, the formation consists of fineto medium-grained sandstone whose mineral constituents are dominated by hard, abrasive minerals such as quartz and feldspar. Significant clay minerals ranging from clay-size to slit- and finesand size particles are also present in the formation.

One of the most significant characteristics of the Vaqueros Formation as relates to shaft tunnel construction is the presence of hard, abrasive minerals. Published literature indicates quartz content of 50 percent to more than 90 percent. Grain size analyses on samples of the sandstone indicate the presence of significant clay-, slit- and fine-sand size particles, ranging from 10 to 25 percent passing the No. 200 sieve. Much of this fraction of "fine" material, however, includes quartz and feldspar.

The structure of the Vaqueros Formation includes both bedding planes and tow orthogonal sets of joints. As shown in Figure 5, the bedding dips steeply into the slope (north-northeast) at angles of 50 to 78 degrees. The nature of the bedding planes is highly variable which typifies sedimentary deposits. In general, the bedding planes are characterized by very thing (< 1mm) partings containing silt and clay. Elsewhere, the silt and clay can be absent entirely.

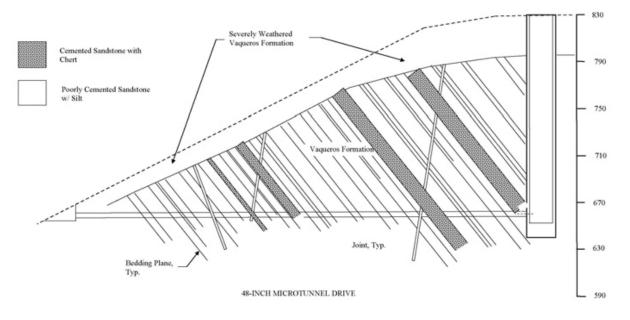
Joints are typically widely spaced, tight, and contain either precipitates of calcium carbonate ("healed") or evidence of weathering. Joints dip steeply from near vertical to between 45 to 80 degrees.

The intact strength of the rock varies between extremely weak and weak to moderately strong. Measured rock strength typically ranges from several hundred to several thousand pounds per square inch (psi), although lower and higher strengths were measured. The lowest measured uniaxial compressive strength was about 10 psi, while the largest measured values were on the order of 7,000 psi. Ground water levels at the site will vary substantially with seasonal variation in precipitation within the watershed and with reservoir levels.

#### Shaft Design

Evacuation for the intake shaft will extend approximately 180 ft  $\pm$  below ground surface and will require excavation through a combination of fill, residual soil and weather rock, and weak to moderately strong, intact rock. Plans for the project require a finished interior shaft diameter of 16- to 20-feet, with an excavation shaft diameter to be determined by the general contractor based on structural requirements shown in the contract documents and means and methods used for initial support.

Design of initial support for the shaft



Source: Black & Veatch Corporation, Intake GBR, 2007 Figure 5. Geologic Section through NWP Intake

excavation will be the responsibility of the contractor. Initial support may consist of linear plate and steel ribs, steel ribs with timber lagging, slurry panels walls, secant piles, or casing installed using drilling methods, such as blind auger drilling. Since slurry panel walls, secant piles, and blind auger drilling provide presupport of the ground prior to substantial excavation prior to installation, including approved management of ground water inflows and significant reductions in excavation volumes.

Due to the unfavorable orientation of bedding planes within the Vaqueros Formation and the generally weak nature of the rock, the material is expected to squeeze and fast ravel within a cycle of bench excavation and erection of initial support. Thus, excavation volume 35 percent greater than those corresponding to a "neat" line would be anticipated for support systems requiring excavation prior to installation. In addition, for linear plate and/or steel rib installations, ground water inflows and corresponding requirements for treatment and disposal must be considered. Based on our analyses, sustained ground water inflows of 500 gpm should be anticipated. Flush flows of up to 2,000 gpm are possible and if encountered are to be grouted to reduce total flows into the shaft to a 500 gpm threshold.

#### Microtunneling

A single 530-foot long microtunneled intake tunnel will connect the shaft to the lake with a lake tap. The construction

method will involve jacking a steel pipe casing following a microtunnel boring machine (MTBM), with the casing serving as initial support and final linear. Plans for the project require a finished intake tunnel diameter of 48- to 72-inches, with the actual diameter to be determined by the general contractor based on MTBMs available at bid time and allowable jacking space resulting from the selected shaft diameter. The contractor will be responsible for selection of the appropriate MTBM and casting thickness to carry the thrust of the jacking forces and other loads.

The MTBM will be driven from the shaft to the reservoir and retrieved "in the wet." The retrieval of the MTBM will be staged from an excavation into the slope of the reservoir side wall that will be prepared prior to initiation of microtunneling.

The MTBM will be a closed, pressurized face, steerable, laser-guided, articulated capable tunnel shield of exerting continuous, controlled pressure at the tunnel fact to prevent uncontrollable groundwaterinflowsandgroundmovement into the cutter chamber, with a reversible cutterhead drive system to minimize rotation of pipe during installation. It will also be capable of handling the various anticipated ground conditions to minimize loss of ground during tunneling and steerable and capable of controlling the advance of the heading to maintain line and grade within the specified tolerances. It will include a system to inject lubricant over and around the rear of the MTBM to reduce jacking friction and a slurry system

to balance ground and groundwater pressure up to 140 feet of hydrostatic head.

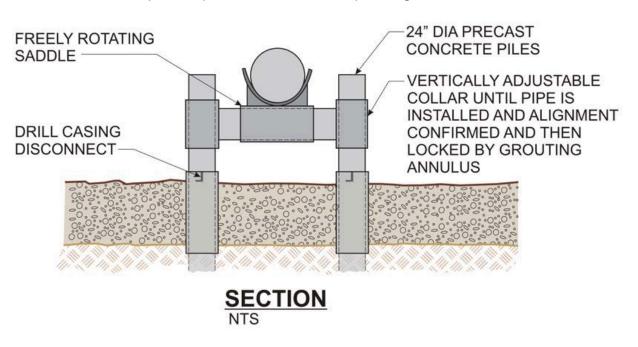
The overall tunneling system will also include a casing jacking system; launch seal affixed to the shaft wall and through which the MTBM and steel pipe passes; equipment to maintain proper air quality in case the contractor selects manned microtunnel operations during construction; lighting fixtures in watertight enclosures; and possible air lock to assist in cutter changes where changing cutters under atmospheric pressure is infeasible. The steel casing pipe will be either all welded steel pipe or Permalok pipe with gasketed joints.

The final push of the MTBM into the reservoir, the "lake tap," is expected to be

the riskiest part of the job. During the lake tap operations, the contractor's principal focus will be on the safety of the work and personnel. The contractor will select its means and methods for performing the lake tap, including the type of removal bulkheads and/or flood valves to be used to ensure that the work is protected from flooding and unexpected water inflows given the relatively high head working conditions. The sequence for temporary support and removal of the MTBM to the lake surface will also be a critical activity.

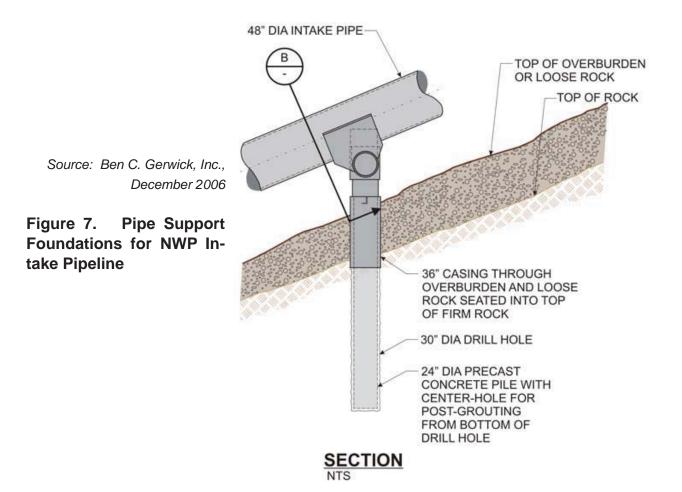
## Sloping Intake and Intake Ports

The seven-port sloping intake was selected to maximize the District's ability to withdraw water from the best locations depending on actual reservoir water



Source: Ben C. Gerwick, Inc., December 2006

# Figure 6. Section through Intake Pipeline Saddle Support



surface elevation, and time of year. The inlet ports and uniformly spaced vertically at 20-foot centers. The intake consists of a 400-foot long, 48-inch diameter, free-standing pipeline anchored on pipe supports; 24-inch diameter inlet ports with isolation butterfly valve and screen; and hydraulic system for intake valve operation.

A key construction planning activity focused on how to quickly and efficiently install the pipe supports and pipeline segments underwater from the lake surface. As shown in Figures 6 and 7, the intake design team, including underwater specialists from Ben C. Gerwick, Inc., devised a pipe support system that involves construction of cased drilled holes, followed by insertion of precast concrete piers and subsequent grouting to solidly lock the piers in-place. The individual pipeline segments (50 foot each) will be connected into 100- to 150foot lengths and lowered down onto the pipe supports and connected to the pier tops with a fabricated steel pipe saddle. The saddle details will be finalized prior to advertisement for bidding, and the means and methods and final installation

sequencing will be left to the installing contractor.

## Marine Construction

Marine construction activities will support the installation of the sloping intake and retrieval of the MTBM. Associated with the marine construction will be the replacement / relocation of the existing log boom in the intake / spillway area.

Marine construction will involve establishing a floating marine operation with barge/crane and access to shore; divingoperations;fueltransfer;underwater excavation; placement of tremie concrete; underwater construction of pile supports; and underwater placement of pipeline segments, valves and screens.

# Summary / Lessons Learned

Although construction cost was a key

factor, the NWP Intake design evolution was eventually driven by water quality requirements – to provide an intake with seven ports that will allow water to be drawn from various depths of the reservoir for optimal water quality control. As a result, a surface-mounted sloping intake was adopted.

With this change in concept, underwater construction and placement of the intake pipe became a key focus of the design team. Details of construction sequencing and the design of pipe supports that are adjustable underwater were developed to assure the project is constructed in a safe and timely manner.

Advertisement to bid for the NWP Intake will occur in Spring 2007, and construction is scheduled to commence in late 2007.

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Black & Veatch Corporation, Nacimiento Water Project Technical Memorandum (TM) 8, Water Quality Investigations, January 2006 (Final).

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Marine Research Specialists, Nacimiento Water Project, Environmental Impact Report (Final), December 2003, prepared for the San Luis Obispo County Department of Planning and Building

# NOTES