# Draft

# **Groundwater Sustainability Plan Chapter 6 – Water Budget**

for the

# Arroyo Grande Valley Groundwater Subbasin Groundwater Sustainability Agencies



Prepared by



8/11/2021

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# **APPENDICES**

# LIST OF TERMS USED

**Abbreviation**AB

Assembly Bill

ADD Average Day Demand

AF Acre Feet

AFY Acre Feet per Year
AMSL Above Mean Sea Level

Basin Plan Water Quality Control Plan for the Central Coast Basin

Cal Poly California Polytechnic State University

CASGEM California State Groundwater Elevation Monitoring program

CCR California Code of Regulations

CCRWQCB Central Coast Regional Water Quality Control Board

CCGC Central Coast Groundwater Coalition
CDFM Cumulative departure from the mean
CDPH California Department of Public Health

CIMIS California Irrigation Management Information System

City City of San Luis Obispo
County County of San Luis Obispo

CPUC California Public Utilities Commission

CPWS-52 Cal Poly Weather Station 52

CRWQCB California Regional Water Quality Control Board

CWC California Water Code
DDW Division of Drinking Water
Du/ac Dwelling Units per Acre

DWR Department of Water Resources
EPA Environmental Protection Agency
ERMWC Edna Ranch Mutual Water Company

ET<sub>0</sub> Evapotranspiration

EVGMWC Edna Valley Growers Ranch Mutual Water Company

°F Degrees Fahrenheit FAR Floor Area Ratio FY Fiscal Year

GAMA Groundwater Ambient Monitoring and Assessment program

GHG Greenhouse Gas

GMP Groundwater Management Plan

GPM Gallons per Minute

GSA Groundwater Sustainability Agency
GSC Groundwater Sustainability Commission

GSP Groundwater Sustainability Plan
GSWC Golden State Water Company

IRWMP San Luis Obispo County Integrated Regional Water Management Plan

kWh Kilowatt-Hour

LUCE Land Use and Circulation Element LUFTs Leaky Underground Fuel Tanks

MAF Million Acre Feet

MCL Maximum Contaminant Level

**Abbreviation Definition** MG Million Gallons

MGD Million Gallons per Day Mg/L Milligrams per Liter

MOA Memorandum of Agreement
MOU Memorandum of Understanding

MWR Master Water Report

NCDC National Climate Data Center

NOAA National Oceanic and Atmospheric Administration

NWIS National Water Information System

RW Recycled Water

RWQCB Regional Water Quality Control Board

SB Senate Bill

SGMA Sustainable Groundwater Management Act
SGMP Sustainable Groundwater Management Planning

SGWP Sustainable Groundwater Planning

SLO Basin San Luis Obispo Valley Groundwater Basin

SLOFCWCD San Luis Obispo Flood Control and Water Conservation District

SCML Secondary Maximum Contaminant Level

SOI Sphere of Influence

SNMP Salt and Nutrient Management Plan

SWRCB California State Water Resources Control Board

TDS Total Dissolved Solids

TMDL Total Maximum Daily Load

USGS United States Geological Survey

USFW United States Fish and Wildlife Service

USTs Underground Storage Tanks
UWMP Urban Water Management Plan

UWMP Act Urban Water Management Planning Act

UWMP Guidebook Department of Water Resources 2015 Urban Water Management Plan Guidebook

VRMWC Varian Ranch Mutual Water Company

WCS Water Code Section
WMP Water Master Plan
WPA Water Planning Areas
WRF Water Reclamation Facility

WRCC Western Regional Climate Center WRRF Water Resource Recovery Facility

WSA Water Supply Assessment WTP Water Treatment Plant

WWTP Wastewater Treatment Plant

# **EXECUTIVE SUMMARY**

This section to be completed after GSP is complete.

# 6 WATER BUDGET (§ 354.18)

The purpose of a water budget is to provide an accounting and assessment of the total annual volume of groundwater and surface water entering and leaving the Subbasin, including historical, current, and projected water budget conditions, and the change in volume stored. Both numerical and analytical methods have been used during water budget preparations for the GSP. The analytical method as used in this document refers to application of the water budget equation and the inventory method using spreadsheets, with groundwater flow estimates based on Darcy's Law and change in storage calculations based on the specific yield method.

Numerical methods refer to surface water and groundwater flow modeling, which provide a dynamic and more rigorous analysis of both surface-groundwater interactions and the impacts from pumping on groundwater in storage. The historical and current analytical groundwater budget will be used as part of the Subbasin Hydrogeologic Conceptual Model (HCM) to prepare input estimates and provide a check for the numerical model, from which the projected water budget will be produced. This chapter presents the analytical water budget for the historical and current periods and the numerical model water budget for the projected future period. Once the numerical model water budget is calibrated, the results will be presented as comparisons to the analytical water budget.

A water budget identifies and quantifies various components of the hydrologic cycle within a user-defined area, in this case the Arroyo Grande Valley groundwater Subbasin. Water circulates between the atmospheric system, land surface system, surface water bodies, and the groundwater system, as shown in Figure 6-1 (DWR, 2016). The water budget equation used for the analytical method is as follows:

#### INFLOW - OUTFLOW = CHANGE IN STORAGE

Inflow is the sum of all surface water and groundwater entering the Subbasin and outflow is the sum of all surface water and groundwater leaving the Subbasin. The difference between total inflow and total outflow over a selected time period is equal to the change in total storage (surface water and groundwater) within the Subbasin over the same period. Components of inflow and outflow represented in the water budget are shown in Figure 6-2. Not all of the components shown are needed for the Subbasin GSP. A key using letters to represent components in this water budget has been added to Figure 6-2 for reference with the main water budget tables. Some components have been modified and renamed from the original DWR figure to better represent this specific water budget.

The water budget equation given above is simple in concept, but it is challenging to measure and account for all the components of inflow and outflow within a Basin. Some of these components can be measured or estimated independently, while others are calculated using the water budget equation. The water budget for this GSP has been prepared for the Subbasin as a whole.

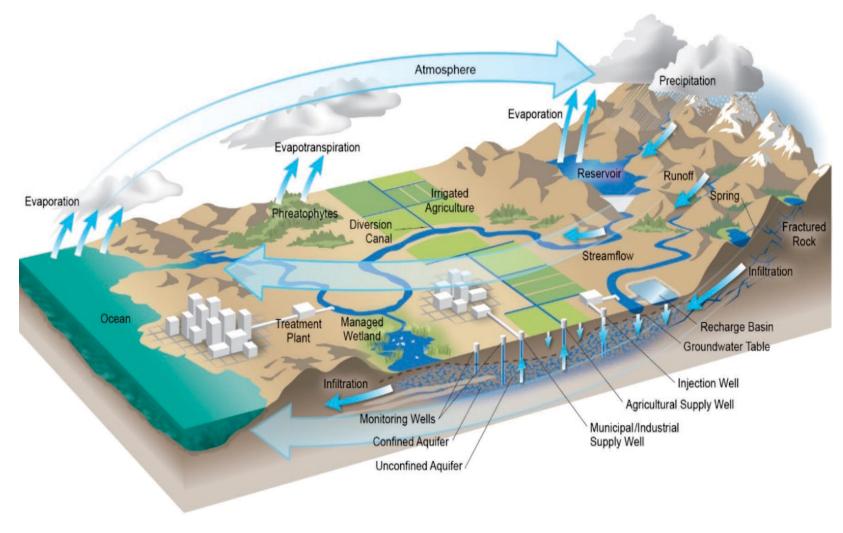


Figure 6-1: The Hydrologic Cycle.

Source: Department of Water Resources (Water Budget BMP, 2016)

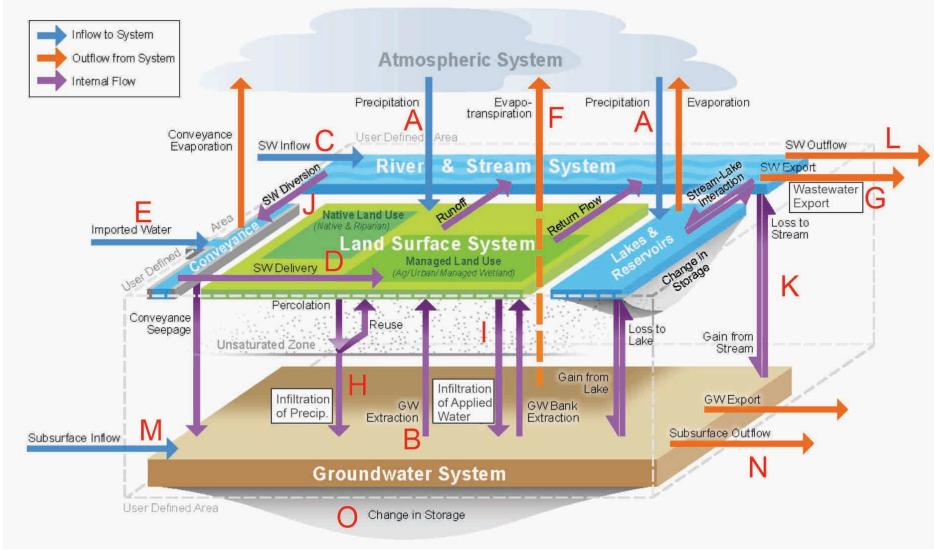


Figure 6-2: Components of the Water Budget.

Source: Modified from Department of Water Resources (Water Budget BMP, 2016)

The Subbasin is approximately 2,900 acres (4.5 square miles), and receives surface inflow from a watershed of approximately 65,800 acres (102.8 square miles) of which approximately 44,000 acres (68.8 square miles; 67%) are upstream of Lopez Dam. The largest tributary to Arroyo Grande Creek entering the Subbasin downstream of the dam is Tar Spring Creek (Figure 3-3; Chapter 3).

Table 6-1 presents the historical surface water and groundwater budgets for the Subbasin. Bar graphs for the surface water and groundwater budgets are included in Figure 6-3 and Figure 6-4. A letter key has been added to provide a visual reference between Table 6-1 and Figure 6-2.

Note that Figure 6-2 separates the water budget into four components (atmospheric system, land surface system, river & stream system, and groundwater system). The atmospheric system transfers evaporation to precipitation and overlies the other systems. The land surface system is the portion of the water budget that includes land surface and the unsaturated zone extending to the top of the groundwater system. The rivers & streams system is the portion of the water budget that includes rivers, streams, conveyance facilities and diversion ditches, and lakes and reservoirs. The atmospheric, land surface, and river & streams water budgets for this Subbasin have been combined into a single surface water budget. As a result, not all the components in Figure 6-3 have corresponding budget items listed for the Subbasin. For example, the runoff and return flow components of the land surface system into the river & stream system in Figure 6-2 are part of the surface water outflow component (Labeled "L").

The bar graphs are graphical representations of the water budget that allow quick comparisons of the various budget quantities. Figure 6-3 illustrates the surface water budget portions of Table 6-1, while Figure 6-4 illustrates the groundwater budget portions of the table. Water budget climate, historical time period, methodology, sustainable yield, and overdraft interpretation are also presented in this chapter.

Table 6-1: Historical Water Budget – Arroyo Grande Subbasin.

																		yo Grand r <b>ande S</b>												
		SUR	FACE W	/ATER I	NFLOW	/ (AF)									FLOW		0,00	unac s	Jubbus		ROUN	DWAT	ER INF	LOW (	AF)	GROUNE	WATER (	OUTFLO	W (AF)	
Water Year	Precipitation	GW extractions (Urban)	GW extractions (Ag)	Stream Inflow	Surface Water Delivery	bed	TOTALIN	ET of Precipitation	ET of Applied Water (Urban)	ET of Applied Water (Ag)	Riparian ET	Wastewater export	of on	lied an)	ਰ	Surface Water Diversion	GW-SW interaction	Stream outflow	TOTAL OUT	Infiltration of Precipitation	þ. (	Infilt. of Applied Water (Ag)		Subsurface Inflow	TOTALIN	GW Extractions (Urban)	GW Extractions (Ag)	Subsurface Outflow	TOTAL OUT	Change in GW Storage (AF)
KEY	Α	В	В	С	D	E		F	F	F	F	G	Н	I	I	J	K	L		Н	I	I	K	M		В	В	N		0
1988	2,380	120	2,810	3,950	450	440	10,150	2,260	480	2,170	230	180	100	70	640	450	1,350	2,220	10,150	100	70	640	1,350	170	2,330	120	2,810	480	3,410	-1,080
1989	3,050	110	2,410	4,310	450	410	10,740	2,710	450	1,850	230	160	280	70	560	450	1,470	2,510	10,740	280	70	560	1,470	170	2,550	110	2,410	480	3,000	-450
1990	1,920	110	2,810	4,150	450	390	9,830	1,900	430	2,170	230	150	20	70	640	450	1,420	2,360	9,840	20	70	640	1,420	170	2,320	110	2,810	480	3,400	-1,080
1991	3,920	110	2,590	4,110	450	380	11,560	3,020	430	1,990	230	150	740	70	590	450	1,410	2,490	11,570	740	70	590	1,410	170	2,980	110	2,590	480	3,180	-200
1992	4,150	110	2,530	5,040	450 450	390	12,670 16,750		430 420	1,940	230	150	880	70 70	580 550	450	1,730	3,130	12,670	880	70	580 550	1,730	170	3,430	110	2,530	480	3,120 2,990	310 830
1993 1994	5,820	110	2,400	7,590	450	380		3,150 2,740	400	1,840	230	150	2,210		510	450	820	6,840	16,730		70		820	170 170	3,820	110	2,400	480 480		-450
1995	3,080 7,600	110 110	2,150 2,340	3,700 11,900	450	360 380	9,850 22,780	3,160	420	1,640 1,800	230 230	140 150	290 3,400	60 70	540	450 450	1,260	2,120 11,270	9,840 22,780	290	60 70	510 540	1,260	170	2,290 5,470	110 110	2,150 2,340	480	2,740 2,930	2,540
1996	4,610	110	2,320	19,490	450	420	27,400	3,170	460	1,780	230	170	1,190	70	540	450	1,110		27,410		70	540	1,110	170	3,080	110	2,340	480	2,910	170
1997	6,010	120		55,830		370	65,510		420	2,110	230	150	2,350	70	620	450	950				70	620	950	170	4,160	120	2,730	480	3,330	830
1998	9,070	110		80,040		420	92,150		460	1,570	230	170	1,310	70	490	450		82,860			70	490	1,370	170	3,410	110	2,060	480	2,650	760
1999	3,670	120		17,350	450	450	24,860		490	2,190	230	180	560	80	640	450	990				80	640	990	170	2,440	120	2,820	480	3,420	-980
2000	4,520	110	2,020	9,070	450	430	16,600		470	1,540	230	170	1,080	70	480	450	520	8,400	16,600		70	480	520	170	2,320	110	2,020	480	2,610	-290
2001	4,830	110	2,100	10,420	450	450	18,360		490	1,610	230	180	1,340	70	490	450	590	9,710	18,350	_	70	490	590	170	2,660	110	2,100	480	2,690	-30
2002	2,970	120	2,400	6,530	450	450	12,920	2,700	490	1,850	230	180	230	80	550	450	1,340	4,830	12,930		80	550	1,340	170	2,370	120	2,400	480	3,000	-630
2003	4,070	120	1,960	8,350	450	460	15,410	3,100	490	1,490	230	180	790	80	470	450	900	7,210	15,390	790	80	470	900	170	2,410	120	1,960	480	2,560	-150
2004	3,060	120	2,110	7,200	450	430	13,370	2,740	480	1,610	230	170	260	80	490	450	780	6,080	13,370	260	80	490	780	170	1,780	120	2,110	480	2,710	-930
2005	3,850	120	2,020	11,060	450	420	17,920	3,030	470	1,550	230	170	670	80	480	450	2,270	8,540	17,940	670	80	480	2,270	170	3,670	120	2,020	480	2,620	1,050
2006	5,950	130	2,030	8,990	450	450	18,000	3,190	500	1,550	230	180	2,280	80	480	450	510	8,550	18,000	2,280	80	480	510	170	3,520	130	2,030	480	2,640	880
2007	1,790	160	2,670	5,780	450	430	11,280	1,790	500	2,070	230	170	0	80	600	450	630	4,760	11,280	0	80	600	630	170	1,480	160	2,670	480	3,310	-1,830
2008	4,810	160	2,660	7,500	450	400	15,980	3,190	480	2,060	230	160	1,320	80	600	450	430	6,980	15,980	1,320	80	600	430	170	2,600	160	2,660	480	3,300	-700
2009	2,710	150	2,410	5,450	340	370	11,430	2,550	440	1,880	230	150	140	80	530	340	1,120	3,980	11,440		80	530	1,120	170	2,040	150	2,410	480	3,040	-1,000
2010	5,340	150	1,770	6,310	580	360	14,510		430	1,320	230	150	1,760	80	450	580	1,290	5,020	14,510		80	450	1,290	170	3,750	150	1,770	480	2,400	1,350
							18,730											7,210		3,110					5,640	150	2,020	480	2,650	
2012	2,830	160					11,250							80	530			3,760			80		1,060		2,010	160	2,130	480	2,770	
	3,030			5,380			12,290							80	640			3,950			80		1,100		2,240	170	2,840	480		-1,250
	1,420			4,220			9,090			1,940			0	80	560			2,410	1		80		1,440		2,250	170	2,500	480	3,150	
	2,130			3,620			9,340			2,060			50	80	590			2,030	1		80		1,240	170	2,130	170	2,650	480		-1,170
	3,870			4,700			11,600			1,620			640	80	480			2,870	1		80		1,610	170	2,980	170	2,100	480	2,750	
	6,860						18,270							90	560			5,500			90		2,730		6,550	180	2,500	480	3,160 2,900	
	2,170						10,110						50	90	500			3,600			90	500		170	1,630	190	2,230	480		
	4,920			7,560		220	15,230 10,230	2.760	420	1,320	230	120	210	90 90	420 530			6,200 2,780			90 90		1,290 800		3,280 1,800	180 200	1,940 2,340	480 480	2,600	680 -1,220
	3,020							2,700	440	1,010	230	130	210	30	330	430	800	2,700	10,230	210	30	330	800	1/0	1,000	200	2,340	400	3,020	1,220

Type Year: Dry / Below Normal / Above Normal / Wet

AF = Acre-Feet; KEY = Reference Components on Figure 6-2

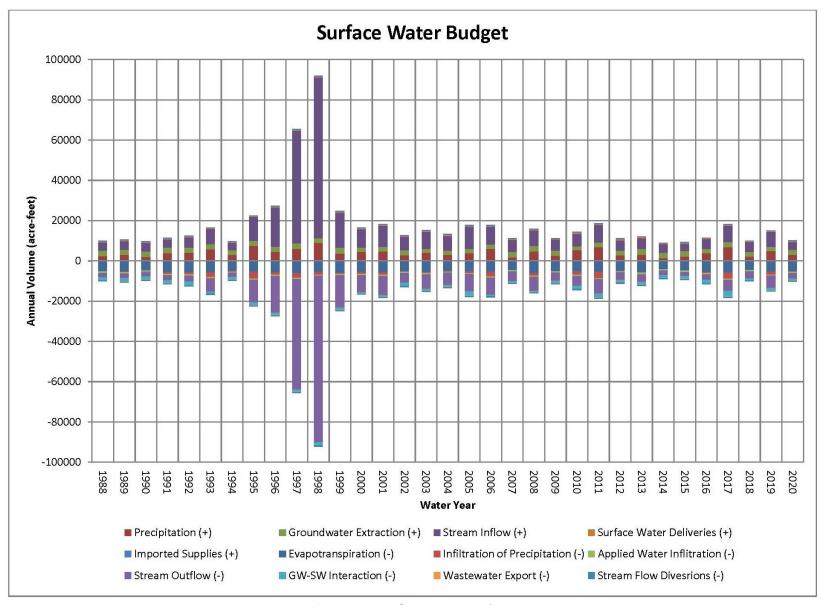


Figure 6-3: Surface Water Budget.

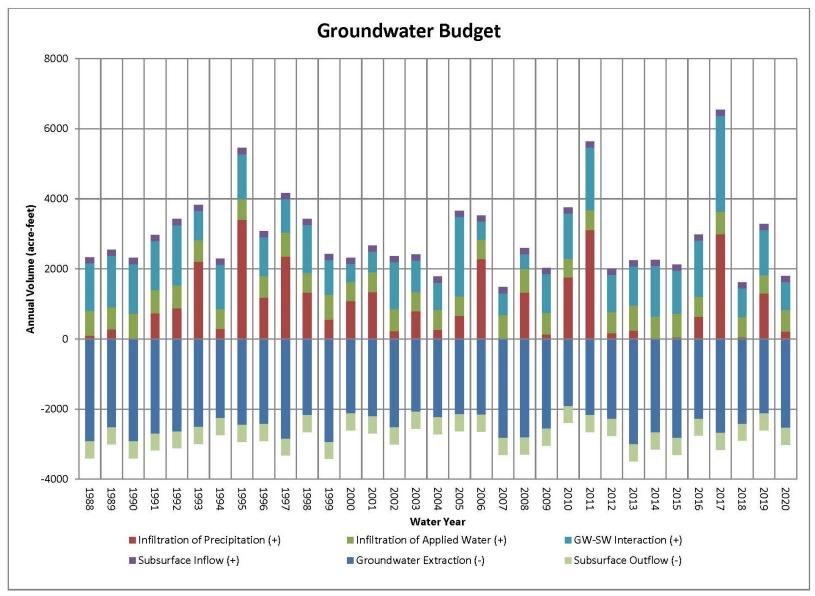


Figure 6-4: Groundwater Budget.

# **6.1 CLIMATE**

Climate is one of the principal measures of water supply conditions and is used for hydrologic base period definition and for developing evapotranspiration estimates. The main component of climate monitoring in the Subbasin is rainfall, with records at Lopez Dam (Station 737; formerly Station 178.1) beginning in the 1968-69 rainfall year (July 1st – June 30th). Rainfall is used in the water budget for establishing the hydrologic base period needed for representing long-term water supply conditions.

Another climate parameter used in the water budget is evapotranspiration. Evapotranspiration is calculated from a combination of monitored parameters, such as air temperature, wind speed, solar radiation, vapor pressure, and relative humidity. These parameters, along with precipitation, have been monitored at CIMIS Station #52 (San Luis Obispo – Cal Poly) since 1986. The water budget uses crop evapotranspiration for estimating the applied irrigation requirements for crops (see Section 6.3.4.2). Cal Poly is within DWR reference evapotranspiration Zone 6 (Upland Central Coast), which is one of 18 climate zones in California based on long-term monthly average reference evapotranspiration (CIMIS, 1999). Approximately one third of the Subbasin is within Climate Zone 6, with the remaining two thirds in Climate Zone 3 (Coastal Valleys). CIMIS Station #202 (Nipomo) is within Climate Zone 3, with a record that begins in 2006. A correlation between evapotranspiration at CIMIS Stations #52 and #202 was performed to extend a record representative of Climate Zone 3 to the beginning of the historical base period, as discussed below.

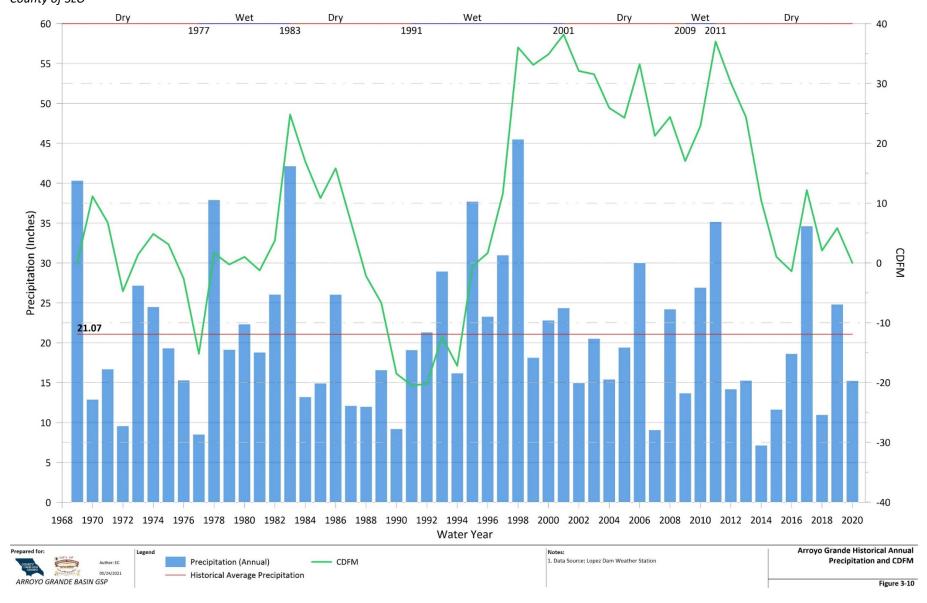
# 6.1.1 Historical Climate/Base Period

The historical rainfall record at Lopez Dam has been used to define a period of years, referred to as a base period, which represents long-term hydrologic conditions. As described by DWR (2002):

The base period should be representative of long-term hydrologic conditions, encompassing dry, wet, and average years of precipitation. It must be contained in the historical record and should include recent cultural conditions to assist in determining projected Basin operations. To minimize the amount of water in transit in the zone of aeration, the beginning and end of the base period should be preceded by comparatively similar rainfall quantities.

The historical rainfall record for the Lopez Dam Station was presented in Figure 3-10; Chapter 3. The SLOCFCWCD reports rainfall data on a water year basis running from July 1 through June 30 (also referred to as rainfall year), while stream flow data is reported from October 1 through September 30 (San Luis Obispo County, 2005). The DWR reports hydrologic data on a water year basis from October 1 through September 30. These conventions are maintained for the water budget, and the DWR water year is used for all water budget components of inflow and outflow. Water years are referenced herein based on the ending year.

The hydrologic base period selected to represent historical climatic conditions for the Subbasin encompasses the years 1988 through 2020 (33 years). Average precipitation at Lopez Dam over this base period was 20.9 inches, compared to the long-term average of 21.07 inches, and included wet, average, and dry periods (Figure 6-5). These periods are visually defined by the movement of the cumulative departure from mean precipitation curve, which declines over dry periods, is flat through average periods, and rises over wet periods.



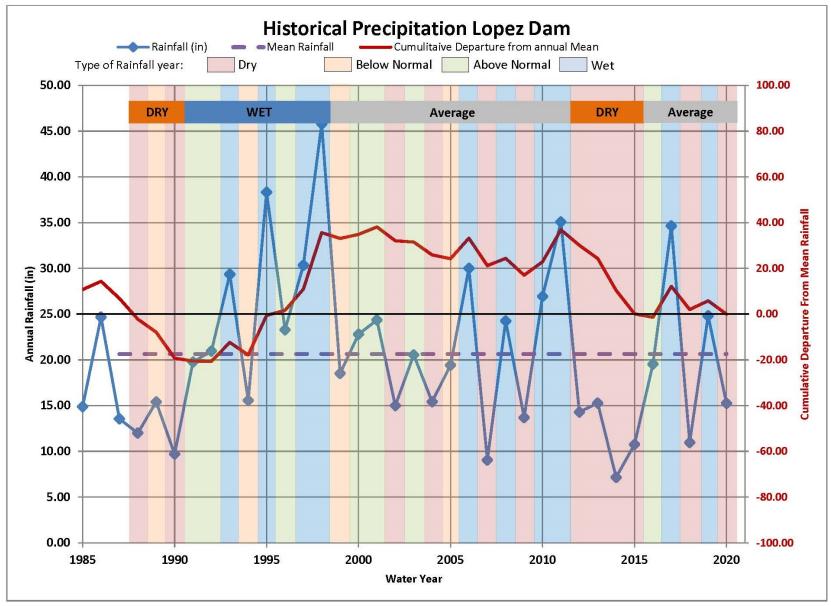


Figure 6-5: 1988-2020 Historical Base Period Climate.

Water year types for this water budget have been developed and classified based on annual precipitation as a percentage of the previous 30-year average precipitation. Each July 1 through June 30 rainfall year of the historical base period was given a ranking of 1 (wettest) through 30 (driest) based on a comparison to a 30-year (rolling) data set. The minimum precipitation threshold for wet type years was assigned based on the average for the 10th ranked year (23.75 inches). The maximum precipitation threshold for dry type years was assigned based on the average for the 21st ranked year (15.05 inches). Below normal (from 15.05 to less than 19.66 inches) represents the 16th through 20th ranked years, while above normal (from 19.65 to 23.75 inches) represents the 10th through 15th ranked years. Note that the division between below normal and above normal rainfall (19.66 inches) is less than the average over the base period (20.9 inches) because there are more below average rainfall years than above average years. The water year types were developed from Lopez Dam rainfall records. The rainfall thresholds for water year types are summarized in Table 62.

Water Year Type	Rainfall Threshold (in.)*
Dry	<15.05
Below Normal	15.05 - <19.66
Above Normal	19.66 – 23.75
Wet	>23 75

Table 6-2: Rainfall Thresholds for Water Year Types.

The base period includes recent cultural conditions (i.e. water supply, water demand, and land use) as recommended. Differences between water in transit in the vadose zone (deep percolation of precipitation and stream seepage) are minimal, based on comparing the two rainfall years leading up to the beginning and ending of the base period. The 1986 and 1987 rainfall years leading into the base period have 24.68 inches and 13.56 inches, respectively, compared to 24.82 and 15.25 inches of rainfall at the end of the base period in 2019 and 2020 (Figure 6-5).

An isohyetal map of average annual rainfall is shown in Figure 4-3 (Chapter 4). The average annual precipitation across the Subbasin between 1981 and 2010 ranged from 15.5 inches to 20 inches, and averaged approximately 17 inches.

The water budget uses the Lopez Dam rain gauge (Station 737) to identify the historical base period and water year types due to the extensive period of record. Annual rainfall used in the surface water budget calculations that involve precipitation volumes, however, are adjusted to account for the difference between rainfall at the dam and average rainfall across the Subbasin.

Table 6-3 presents the annual rainfall at Lopez Dam over the historical base period. Water years are listed as dry, below normal, above normal, and wet in accordance with the thresholds described above. Average annual rainfall over the historical base period at the dam is estimated to be 20.9 inches.

<sup>\*</sup>As measured at Lopez Dam

Table 6-3: Historical Base Period Rainfall.

	_	Lopez Dam					
Year	Туре	Rainfall (in.)					
1988	Dry	12.00					
1989	Below Normal	15.40					
1990	Dry	9.70					
1991	Above Normal	19.77					
1992	Above Normal	20.96					
1993	Wet	29.36					
1994	Below Normal	15.57					
1995	Wet	38.34					
1996	Above Normal	23.29					
1997	Wet	30.34					
1998	Wet	45.80					
1999	Below Normal	18.53					
2000	Above Normal	22.80					
2001	Wet	24.36					
2002	Dry	15.00					
2003	Above Normal	20.55					
2004	Below Normal	15.43					
2005	Below Normal	19.43					
2006	Wet	30.02					
2007	Dry	9.05					
2008	Wet	24.26					
2009	Dry	13.70					
2010	Wet	26.93					
2011	Wet	35.08					
2012	Dry	14.30					
2013	Below Normal	15.28					
2014	Dry	7.16					
2015	Dry	10.76					
2016	Below Normal	19.53					
2017	Wet	34.64					
2018	Dry	10.97					
2019	Wet	24.82					
2020	Below Normal	15.25					
	Average	20.9					

#### 6.2 WATER BUDGET DATA SOURCES

The following sources and types of data have been used for the water budget:

- Hydrogeologic and geologic studies and maps
- County stream flow gages
- County and NOAA precipitation Stations
- PRISM 30-year normal dataset (1981-2010)
- CIMIS weather station data
- Aerial Imagery
- County water level monitoring program
- City of Arroyo Grande, County, and DWR land use data and planning documentation
- County Ag Commissioner's Office data sets
- County Water Master Plan
- Stakeholder supplied information
- Water rights filings

## 6.3 HISTORICAL WATER BUDGET

In accordance with GSP regulations, the historical water budget shall quantify the following, either through direct measurement or estimates based on data (reference to location of data in Chapter 6 also listed):

- (1) Total surface water entering and leaving a Basin by water source type (Table 6-1).
- (2) Inflow to the groundwater system by water source type, including subsurface groundwater inflow and infiltration of precipitation, applied water, and surface water systems, such as lakes, streams, rivers, canals, springs, and conveyance systems (Table 6-1).
- (3) Outflows from the groundwater system by water use sector, including evapotranspiration, groundwater extraction, groundwater discharge to surface water sources, and subsurface groundwater outflow (Table 6-1).
- (4) The change in annual volume of groundwater in storage between seasonal high conditions (Table 6-1).
- (5) If overdraft occurs, as defined in Bulletin 118, the water budget shall include a quantification of overdraft over a period of years during which water year and water supply conditions approximate average conditions (Section 6.3.8).
- (6) The water year type associated with the annual supply, demand, and change in groundwater stored (Table 6-1).
- (7) An estimate of sustainable yield for the Basin (Section 6.3.7).

## 6.3.1 Historical Time Period

The time period over which the historical water budget is estimated is the hydrologic base period from 1988-2020 (33 years). Groundwater storage calculations using the specific yield method were performed for Spring 1987, 1990, 1996, 2002, 2009, 2011, 2015, 2017, and 2020. These years include the beginning (Spring 1987) and ending (Spring 2020) storage in the base period, with multiple interspersed years to characterize change in storage trends through the base period.

#### 6.3.2 Historical Land Use

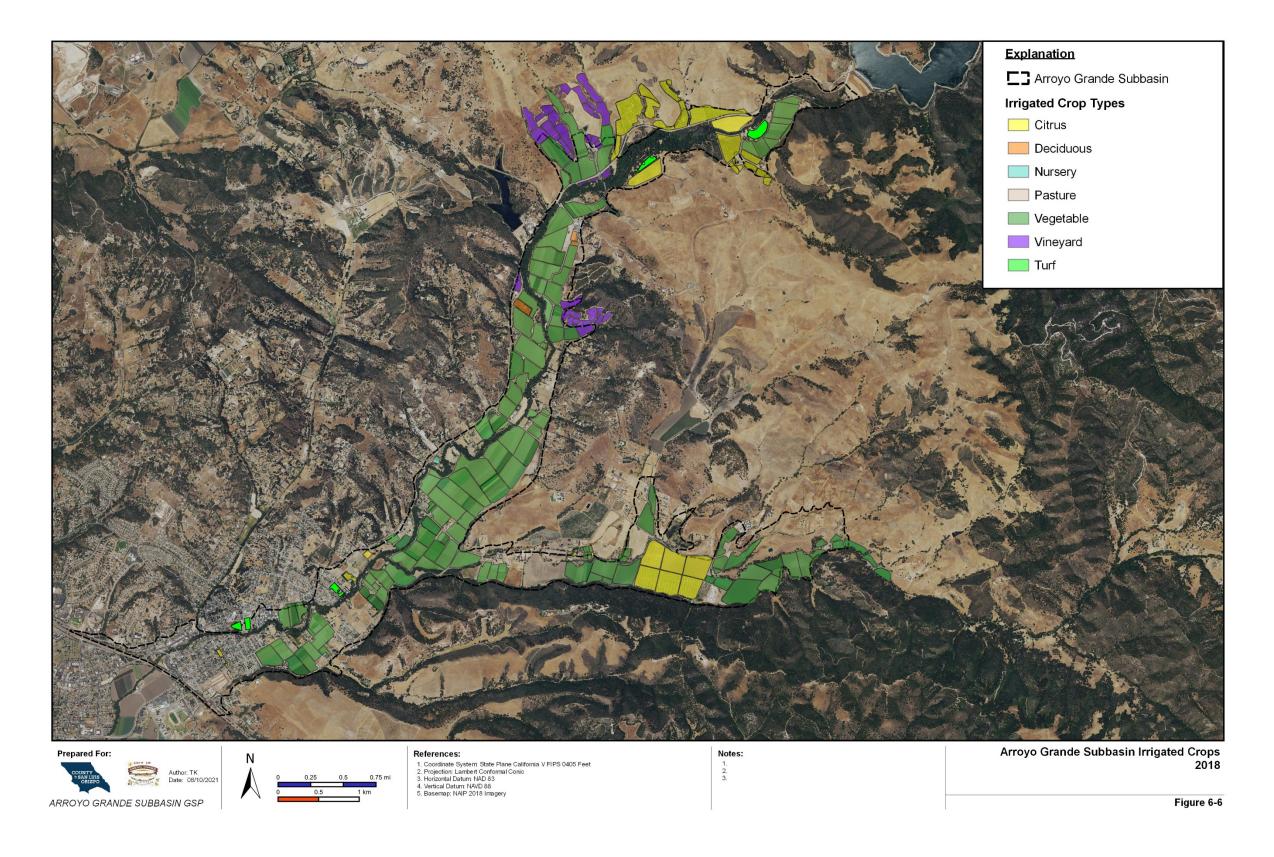
Land use is one of the primary data sets used in developing a water budget. Several types of land use/land cover in the basin have been used to estimate components of the water budget. For example, the acreages of various crops are multiplied by their respective water use factors to estimate agricultural groundwater extractions (Section 6.3.4.2), and acreages of various land covers are multiplied by empirical correlations to

estimate their respective evapotranspiration and percolation of precipitation (Section 6.3.4.1). The land uses/land covers including the following:

- Irrigated Agriculture
  - o Citrus
  - o Deciduous
  - o Pasture
  - o Vegetable
  - o Vineyard
- Native Vegetation
  - o Brush, trees, native grasses
  - o Wetlands/open water (Riparian)
- Urban/Suburban
  - Developed (City, subdivisions)
  - Open space (parks, empty lots)
  - Turf (play fields)

#### Irrigated Agriculture

Irrigated crop acreage was estimated from aerial imagery of the Subbasin for the following years: 1989, 1994, 1999, 2003, 2005, 2007, and 2011. San Luis Obispo County land use data was used for crop acreage from 2013 to 2020. The DWR land use survey for 1985 was also used. Figure 6-6 shows an example of the County irrigated crop data set for 2018.



Irrigated acreage for years in the historical base period without aerial imagery, surveys, or County data were estimated from the nearest available year with data. Acreages for irrigated crops, estimated from aerial imagery and County datasets used to characterize the historical base period are shown in Table 6-4.

Table 0-4. Imgated Agriculture Acreages.															
Cran Tuna	1985	1989	1994	1999	2003	2005	2007	2011	2013	2014	2015	2016	2017	2018	2019
Crop Type	San Luis Valley Subarea (acres)														
Citrus	26	99	132	130	152	152	156	156	156	176	176	192	192	245	262
Deciduous	5	5	5	10	10	10	27	27	30	22	22	1	10	8	18
Pasture	36	15	1	3	3	3	3	3	3	3	3	3	2	2	0
Vegetable	1,508	1,462	1,414	1,356	1,312	1,328	1,309	1,294	1,307	1,238	1,275	1,063	1,130	1,099	1,018
Vineyard	80	64	93	96	127	127	133	128	128	121	124	127	135	111	111
Subtotal	1,654	1,645	1,646	1,594	1,603	1,619	1,628	1,609	1,625	1,561	1,601	1,386	1,469	1,465	1,410

**Table 6-4: Irrigated Agriculture Acreages.** 

# Native Vegetation and Urban Areas

Native vegetation acreages were compiled using data sets from the National Land Cover Database (NLCD), which is derived primarily from satellite imagery. The years for which NLCD coverage is available are 2001, 2004, 2006, 2008, 2011, 2013, 2016, and 2019. Adjustments to the acreages in the NLCD data were performed to reconcile with the agricultural acreages and urban turf areas compiled using the aerial imagery and crop survey data set. Where the NLCD data sets showed less agricultural acreage than the aerial imagery, the native vegetation (brush, trees, grassland) acreage and urban open space was reduced or increased so the total basin acreage remained constant. The estimated acreages for native vegetation and urban areas, along with irrigated agriculture interpolated from Table 6-4, are presented in Table 6-5 below.

2001 2004 2006 2008 2011 2013 2016 2019 **Land cover Arroyo Grande Subbasin (acres)** Native - brush, trees, grassland 513 517 505 491 493 484 542 582 281 283 282 285 Native - Riparian\* 281 281 282 282 Urban - Developed 394 396 399 400 401 404 408 404 Urban - Open Space 102 84 79 86 97 79 134 181 Urban - Turf 10 14 17 17 17 17 17 10 1,599 1,632 1,429 Irrigated Agriculture 1,611 1,621 1,623 1,609 1,516 2,899 2,899 2,899 2,899 2,899 2,899 Subbasin Total 2,899 2,899

**Table 6-5: Land Cover Acreages.** 

#### 6.3.3 Historical Surface Water Budget

The surface water system is represented by water at the land surface within the boundaries of the Subbasin. As previously mentioned, surface water systems for the water budget include the atmospheric system, lakes & streams system, and the land surface system (Figure 6-2).

## 6.3.3.1 Components of Surface Water Inflow

The surface water budget includes the following sources of inflow:

- Local Supplies
  - Precipitation
  - Groundwater extractions

<sup>\*</sup>riparian corridors mapped as wetlands/open water in NLCD imagery

- Stream inflow at Basin boundary
- Surface Water Deliveries
- Groundwater-Surface Water Interactions
- Local Imported Supplies
  - o Lopez Reservoir Water
  - o Groundwater from outside the Subbasin

#### **Precipitation**

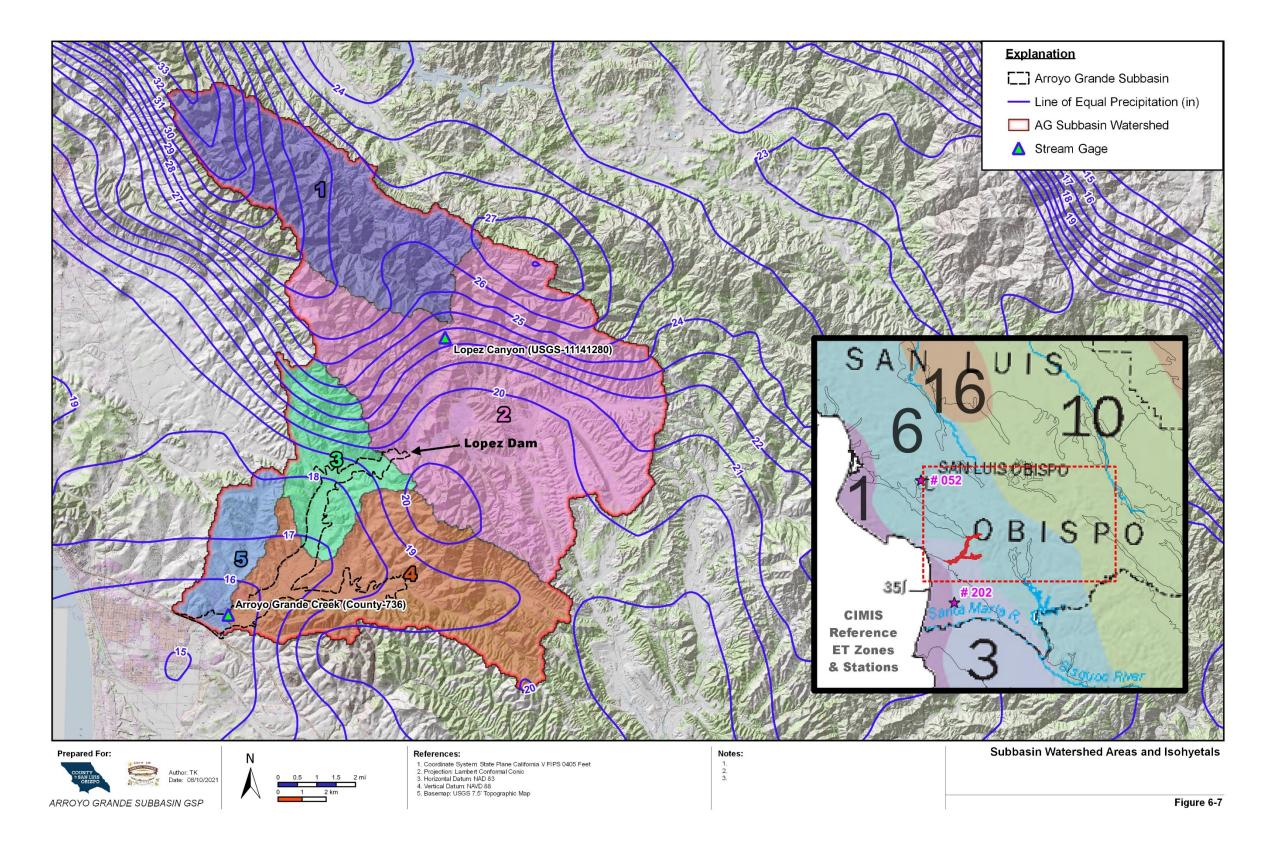
Precipitation occurs as rainfall. The annual volume of rainfall within the Subbasin has been estimated as 80 percent of the rainfall year totals for Lopez Dam, multiplied by the Subbasin area. As previously mentioned, the average rainfall over the subbasin is lower than the average at the Lopez Dam rain station. Rainfall volumes falling within the subbasin boundary are shown as precipitation in the surface water inflow budget of Table 6-1.

#### **Groundwater Extractions**

Groundwater extractions are included in the surface water budget as inflow because after extraction groundwater is distributed and applied at land surface. These extractions are then divided into Urban and Agricultural water use sectors and match the groundwater extraction outflow values from the groundwater budget. Details on data collection and groundwater pumping estimates are provided in the Historical Groundwater Budget section (Section 6.3.3).

# Stream Inflow at Basin Boundary

Inflow along stream channels at the Subbasin boundary has been estimated based on paired watershed methodology. The total watershed area drained by the Subbasin was divided into 5 sub-watershed areas, one of which is the subarea drained by Lopez Canyon (sub-watershed 1, Figure 6-7). Annual (water year) flows from 1988 through 2020 at the Lopez Canyon stream gage was then processed using a watershed area factor and an isohyetal factor to estimate annual flows for each of the other subareas. The watershed area factor was the ratio of the watershed area for which flow was being estimated to the Lopez Canyon gage watershed area. The isohyetal factor addressed differences between the average annual rainfall across each of the sub-watersheds being compared, and consisted of the ratio of average annual precipitation above 13.5 inches between sub-watersheds.



#### **Surface Water Deliveries**

Surface water deliveries represent the movement of water generated by surface water diversion between the streams & lakes system to the land surface system (Figure 6-2). In the surface water budget, in-stream diversions are represented as outflow, and the delivery of this water for irrigation is inflow. They are offsetting values, and further discussed under surface water diversions (Section 6.3.3.2).

#### **Groundwater-Surface Water Interaction (Net)**

Groundwater-surface water interactions take place primarily along stream channels. When groundwater is rising into streams (gaining reaches of a stream), the interaction is a surface water budget inflow and a groundwater budget outflow. Conversely, when stream flow is percolating to groundwater (losing reaches of a stream), the interaction is a surface water budget outflow and groundwater budget inflow. This water budget has combined the gaining and losing stream reaches into single (net) term, the result of which are net losing streams in the Subbasin, which is an outflow component of the surface water budget and inflow component of the groundwater budget. Net groundwater-surface water interaction was estimated by adjusting the percent of stream inflow that recharges groundwater while optimizing the water balance. The optimization consisted of minimizing the sum of squares of the residual error between the calculated change in storage and measured change in storage (Section 6.3.4.1).

#### **Local Imported Supplies**

The City of Arroyo Grande imports water from Lopez Reservoir and also uses groundwater from outside the Subbasin. A portion of the local imported supplies are delivered to customers overlying the subbasin. In order to estimate the volumes of local imported supplies delivered to City residents overlying the Subbasin, the acreages of various City land use classifications (such as Village Core, Single Family Residential Medium Density, and Mixed Use) were multiplied by water use factors for each land use type reported in the Arroyo Grande Urban Water Management Plan (2012, Updated 2015). Local imported supplies are presented in the surface water budget of Table 6-1.

## 6.3.3.2 Components of Surface Water Outflow

The surface water budget includes the following sources of outflow:

- Evapotranspiration of Precipitation
- Evapotranspiration of Applied Water
- Riparian Corridor ET
- Infiltration of Precipitation
- Infiltration of Applied Water
- Wastewater Export
- Surface Water Diversions
- Groundwater-Surface Water Interaction
- Stream outflow (runoff)

# **Evapotranspiration of Precipitation**

The fate of precipitation that falls within the Subbasin boundaries can be divided into three components: evapotranspiration, infiltration, and runoff. Of these three, infiltration has the greatest influence on the groundwater budget and ultimately, Subbasin sustainable yield. Therefore, the approach to estimating the fate of precipitation uses a methodology focused primarily on infiltration, but from which the other two components may also be estimated. This methodology is based on work by Blaney (1933, 1963), and which has been used for other analytical water budgets in major studies of central coast Basins (DWR, 2002; Fugro, 2002).

Evapotranspiration is the evaporation of water from surfaces and the transpiration of water by plants. The first seasonal rains falling on the Subbasin are mostly evaporated directly from surfaces (vegetative canopy, soil, urban area hardscapes) and used to replenish soil moisture deficits that accumulate during the dry season. For the Arroyo Grande – Nipomo Mesa area of the Santa Maria Groundwater Basin, DWR (2002) assumed that precipitation could begin to infiltrate to groundwater (deep percolate) only after 11 inches of annual precipitation had fallen in urban and agricultural irrigation areas, and when 17 inches of rainfall had fallen in areas of native vegetation. In the Paso Robles groundwater Basin, an estimated 12 inches of annual rainfall was needed for infiltration below agricultural lands, while 18 inches of rainfall was needed for infiltration beneath native ground cover and urban/suburban areas (Fugro, 2002).

These threshold values for minimum annual rainfall prior to infiltration are assumed to approximate the annual evapotranspiration of precipitation. Once these thresholds are exceeded, infiltration to groundwater and runoff would become dominant. It is recognized that a portion of the initial annual rainfall may result in runoff, depending on rain intensity, but this is assumed to be offset by the portion of the late season rainfall that is evapotranspired. Since infiltration is the critical component of precipitation with respect to Subbasin sustainable yield, offsetting of early wet season runoff with late wet season evapotranspiration in the water budget is considered a reasonable approach.

The specific thresholds for annual rainfall that are estimated to evapotranspire prior to infiltration and runoff have been developed from Blaney's field studies. Evapotranspiration of precipitation has been estimated by multiplying land use/land cover acreages by the infiltration threshold values. Results of these estimates are shown in the surface water budget of Table 6-1. Additional details of the methodology are provided in section 6.3.4.1 (Components of Groundwater Inflow).

#### **Evapotranspiration of Applied Water**

The evapotranspiration of applied irrigation water has been divided into urban and agricultural sectors. Urban applied water includes residential outdoor irrigation and park/play field irrigation. Most of the urban applied water is from imported local supplies by the City of Arroyo Grande. Other water purveyors within the Subbasin are relatively small (typically less than 30 connections) and are considered rural residential. Estimation of applied water for agricultural irrigation involves a soil-moisture balance approach discussed in section 6.3.4.1 (Components of Groundwater Outflow).

Most water applied for irrigation is taken up by plants and transpired. Some water, however, is lost to evaporation or infiltrates to groundwater as return flow. The evapotranspiration of applied irrigation water has been calculated by subtracting the estimated return flow from the applied water estimates. Both applied water and return flow estimates are presented under the historical groundwater budget section. Results of the calculations of evapotranspiration of applied water are shown in the surface water budget of Table 6-1.

## **Riparian Corridor Evapotranspiration**

Riparian plant communities present along the creeks can access surface flows and creek underflow. An estimated 282 acres of riparian areas are included within the Subbasin (Table 6-5) based on the interpreted NLCD satellite imagery, which maps the riparian corridors as mostly woody wetlands and emergent herbaceous wetlands, with a few acres of open water. Given that the riparian corridor is directly connected to adjacent surface flows, and stream flow is present throughout of the year, water use for the riparian corridor is included in the surface water budget. Riparian vegetation water use is the evapotranspiration of precipitation estimated for the native brush, trees, and grasses land cover, with an additional 0.8 acrefeet per acre of consumptive water use (Fugro, 2002; Robinson, 1958). Riparian evapotranspiration is included in Table 6-1.

#### Infiltration of Precipitation and Applied Water

Infiltration of precipitation and applied water are both outflow components from the surface water budget and inflow components to the groundwater budget. Discussion of these components is provided in Section 6.3.4.1 (Components of Groundwater Inflow).

#### Wastewater Export

When imported surface water is brought into the Subbasin from local supplies (Lopez Reservoir), it is counted as surface water inflow. This imported water is then provided to customers through deliveries from the City of Arroyo Grande. After residential and business use, most of the delivered water that was used indoors is conveyed by sewer out of the Subbasin to a wastewater treatment plant (South San Luis County Sanitation District) for treatment and discharge. Since the wastewater does not return to the Subbasin, it is effectively exported. Similar to the estimated for Local Imported Supplies, the acreages of various City land use classifications (such as Village Core, Single Family Residential Medium Density, and Mixed Use) were multiplied by sewer flow factors for each land use type reported in the Arroyo Grande Wastewater Master Plan (City of Arroyo Grande, 2012) and shown in the surface water budget of Table 6-1.

#### **Stream Flow Diversions**

Stream flow on Arroyo Grande Creek is subject to permitted diversion by in-stream pumping. Reported annual stream flow diversions were compiled from available records, which were considered representative beginning in 2009 (more complete reporting). The reported creek flow diversions ranged from 340 acre-feet in 2009 to 600 acre-feet in 2012, with an annual average diversion of 450 acre-feet per year between 2009 and 2019. The resulting estimated stream inflow estimates for the historical base period are shown in the surface water budget of Table 6-1.

#### **Groundwater-Surface Water Interaction (Net)**

Groundwater-surface water interaction involves both surface water and groundwater budgets. The net interaction is an outflow component for the surface water budget and an inflow component for the groundwater budget (losing streams). Details of the methodology used to develop the groundwater-surface water interaction are presented in the Sections 6.3.4.1 and 6.3.6.

#### **Stream Outflow from Basin**

Stream outflow was estimated using the water balance method and compared to available flow records. No significant changes to surface water in storage are assumed in the water budget from year to year. Storm water runoff exits the Subbasin annually, and creek storage fluctuations are considered minor compared to the total surface water budget. Lopez Reservoir and Lopez Terminal Reservoir are outside of the Subbasin boundary.

Using the water budget equation, stream outflow is estimated as the difference between total surface water inflow and all other components of surface water outflow. Results of stream outflow calculations are presented in Table 6-1. The stream gage on Arroyo Grande Creek at the City of Arroyo Grande (Station 736) is the closest gage to the south Subbasin boundary, and captures runoff from approximately 95 percent of the watershed drained by the subbasin (roughly 65,500 acres gaged out of 68,700 acres of watershed (including watershed area above Lopez Dam).

A comparison of gaged stream flow at Station 736 with the estimated stream flow leaving the basin from the surface water budget is presented in Figure 6-8. The comparison shows that the surface water budget produces stream outflow estimates that are reasonably close to the measured flows at Station 736.

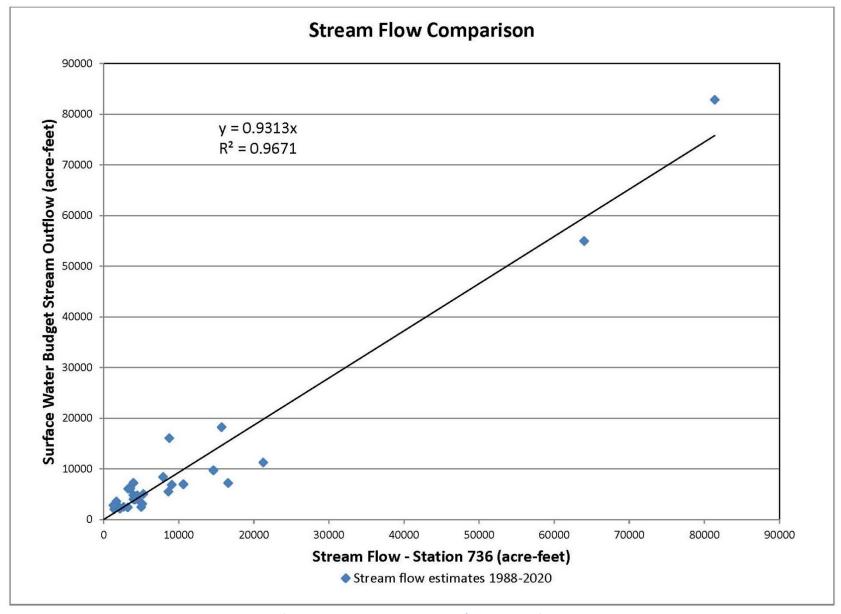


Figure 6-8: 1988-2020 Stream Flow Comparison.

## 6.3.4 Historical Groundwater Budget

The groundwater budget includes the following sources of inflow:

- Infiltration of Precipitation
- Groundwater-Surface Water Interaction
- Subsurface Inflow
- Infiltration of Applied Water (Return Flow)

The groundwater budget includes the following sources of outflow:

- Groundwater Extractions
- Subsurface Outflow
- Groundwater-Surface Water Interaction

#### 6.3.4.1 Components of Groundwater Inflow

#### Infiltration of Precipitation

Infiltration of precipitation refers to the amount of rainfall that directly recharges groundwater after moving through the soil and unsaturated zone (Figure 6-2). Direct measurement of infiltration has not been performed in the Subbasin, and estimates have been prepared based on prior work by Blaney (1933) in Ventura County basins and Blaney et al. (1963) in the Lompoc Area. These studies involved soil moisture measurements at rainfall penetration test plots with various types of land cover, and the resulting deep percolation versus rainfall correlations have been considered applicable to central coast Basins (DWR, 2002; Fugro, 2002). The work by Blaney is several decades old, however, modeling efforts have shown the generalizations are relatively accurate for semi-arid climates (Rosenberg, 2001). The main advantage of Blaney's approach is that it is based on direct measurements of infiltration of precipitation.

Criteria based on Blaney et al. (1963) were used for analytical water budgets in the Santa Maria Valley and Tri-Cities Mesa areas, where it was assumed that precipitation could infiltrate only in urban and agricultural areas when 11 inches of precipitation had fallen annually, and on areas of native vegetation when 17 inches of precipitation had fallen annually. Any amount of rainfall above 30 inches annually was not considered to contribute to deep percolation of precipitation, regardless of the land use classification (DWR, 2002). Correlations between infiltration and annual rainfall based on Blaney (1933) were also used historically for the 2002 Paso Robles Groundwater Basin analytical water budget (Fugro, 2002).

Estimates for infiltration of precipitation for the Arroyo Grande Subbasin have been developed by applying Blaney correlations that restrict deep percolation to precipitation in agricultural areas that occurs after 11-12 inches of rainfall, and in native vegetation areas after approximately 18 inches of rainfall. Native vegetation was the most restrictive land cover for infiltration when tested by Blaney due to high initial soil moisture deficiencies.

Urban areas were not part of the original studies by Blaney. The low permeability of hardscape (buildings and paving) limits infiltration and increases surface evaporation, compared to other types of land cover, but hardscape also increases runoff, which can lead to greater infiltration in adjacent areas receiving the runoff. Therefore, the infiltration threshold was set higher than irrigated agricultural land, but not as high as native grasslands. The Blaney correlation that produces infiltration between irrigated agriculture and native grassland is the curve for non-irrigated grain, with an infiltration threshold of approximately 14 inches of rainfall. Figure 6- plots the data collected by Blaney (1933).

Water Budget (§ 354.18)

As with prior work by the DWR in northern Santa Barbara and southern San Luis Obispo Counties, rainfall above 30 inches was not considered to contribute to deep percolation in the Basin (DWR, 2002). The rainfall values used for the Blaney Correlations in the Subbasin were 80 percent of the rainfall totals at Lopez Dam. Infiltration of precipitation results are shown in Table 6-1.

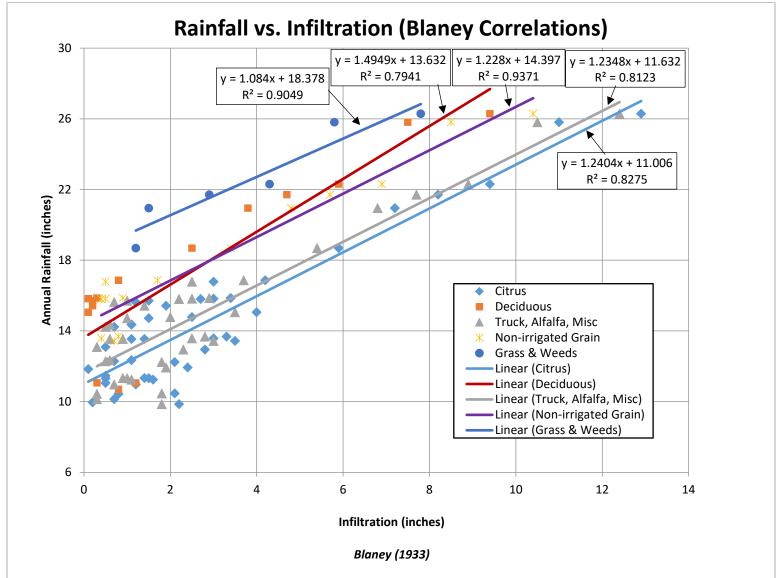


Figure 6-9: Rainfall vs Infiltration.

The land use classifications for which infiltration thresholds have been developed for this GSP include citrus, deciduous, pasture, vegetable, vineyard, native brush/grassland (includes riparian corridors), urban developed/open space, and urban turf. The minimum rainfall needed before infiltration of precipitation can occur for various land uses and covers are summarized in Table 6-6.

Table 6-6: Minimum Rainfall for Infiltration.

Land Use/Cover	Infiltration Threshold (in.)
Citrus	11.0
Deciduous	13.6
Pasture	11.6
Vegetable	11.6
Vineyard	13.6
Native brush/grassland	18.4
Urban developed/open space	14.4
Urban turf	11.6

# **Groundwater-Surface Water Interaction (Net)**

As previously mentioned, groundwater-surface water Interaction involves both components of the surface water and groundwater budgets. The net interaction is an outflow component of the surface water budget and inflow component of the groundwater budget (losing streams).

The groundwater-surface water interaction component is estimated using a mass balance approach for the Subbasin by adjusting the percent of stream inflow that percolates to groundwater (as recharge) while minimizing the sum of squares of the residual error between the calculated change in storage and the measured change in storage (specific yield method) for multiple years. It became apparent during water budget calibration that a variable percentage was needed depending on the type of year (a greater percentage of stream flow percolation during drought years) and reservoir operation (lowest percent of stream flow seepage during reservoir spill years).

The maximum amount of groundwater storage in Subbasin is assumed to be 15,200 acre-feet, based on the specific yield method. In 1998, inflow to the groundwater budget exceeded the maximum storage capacity, and some of the inflow (percolation of precipitation) was transferred to the surface water budget as stream outflow for that year. The groundwater-surface water interaction estimates are in Table 6-1. Additional details of the calibration methodology used to minimize the residual error are presented in Change in Storage (Section 6.3.6).

## Subsurface inflow

Subsurface inflow from bedrock were estimated using Darcy's Law, which is an empirical formula describing the flow of fluid though a porous material, and expressed as:

$$Q = -K \frac{dh}{dl} A$$

Where:

Q = groundwater discharge rate through a cross-sectional area of the porous material

*K* = hydraulic conductivity of the material

 $\frac{dh}{dl}$  = hydraulic gradient at the cross-section

A = cross-sectional area

The negative sign denotes that flow is in the direction of decreasing pressure. Since groundwater pressures are greater within the bedrock hills surrounding the Subbasin than beneath the alluvial valleys, there is subsurface inflow to the Subbasin from bedrock. The application of Darcy's Law to estimate subsurface inflow from bedrock involves simplification and assumptions of uniformity in the subsurface.

Cross-sectional areas for boundary flows were based on the approximate length of the Subbasin boundary (126,500 feet divided into 12 straight-line segments), multiplied by the estimated saturated thickness of Subbasin sediments adjacent to each segment (the weighted average was 70 feet thick). Hydraulic gradients for each segment were developed by averaging topographic slopes between a line along the Subbasin boundary and a line drawn at a 2,500-foot setback from the boundary, and assuming the average hydraulic gradient was approximately three-quarters of these slopes (0.75 ft/ft). The hydraulic conductivity of bedrock was estimated at a nominal 0.03 feet per day. The resulting average annual subsurface inflow from bedrock is 170 acre-feet per year.

#### Infiltration of Applied Water (Return Flows)

Estimates for infiltration of applied water include urban return flow and agricultural return flow. Urban return flow comes from water delivered for domestic or commercial/industrial uses that infiltrates to groundwater, mainly through landscape/turf irrigation and septic system discharges (includes suburban/rural residential return flow). Urban return flow does not include City wastewater that is collected and exported from the Subbasin, which is accounted for in the surface water budget. Agricultural return flows come from applied irrigation water to crops, originating from both groundwater wells and instream diversions.

The first step in estimating urban return flows was to separate delivered water (from local imported supplies and suburban groundwater) into indoor and outdoor use. An estimated 5 percent of indoor use is assumed to be consumptive use (95 percent return flow; EPA, 2008), while 85 percent of outdoor use is consumed (15 percent return flow) based on the typical range of estimates for other local Basins (DWR, 2002; Fugro, 2002). Almost all Indoor water use drains to septic systems or sewer systems. Outdoor water use is generally for irrigation, most of which evapotranspires into the atmosphere.

The distribution of indoor to outdoor water use will vary based on the user. For example, City customers in single-family homes (medium density) are estimated in the Water and Wastewater Master Plans (2015, 2012) to use approximately 700 gallons per day of water and produce 310 gallons per day of wastewater, for an average 44 percent indoor use and 56 percent outdoor use. The indoor and outdoor water use and associated return flows were estimated from water use by the City, suburban/rural residences, and a few commercial operations. Infiltration of Applied Water estimates for urban and agricultural sectors are presented in the historical water budget Table 6-1.

#### 6.3.4.2 Components of Groundwater Outflow

#### **Urban Groundwater Extractions**

Groundwater extraction from wells is the primary component of outflow in the groundwater budget. Estimates for historical pumping were derived primarily from land use data and water duty factors, and from the daily soil-moisture budgets. There are no City groundwater extractions from the Subbasin.

Rural residential groundwater use was estimated based on the number of residences identified on aerial images within the Subbasin but outside of the City water service area. Each rural residence was assigned a water use of 0.8 AFY, consistent with the San Luis Obispo County Master Water Plan (Carollo, 2012) and with stakeholder-provided information. In addition to rural residences overlying the Subbasin, residences in two subdivisions with homes outside of the Subbasin but supplied by alluvial wells in the Subbasin were added to the total count.

Aerial images for multiple years were reviewed for rural residential development. The estimated number of residences outside of the City service area was compiled, and resulting computed rural residential water use for these years is presented in Table 6-7.

Table 6-7. Rafai Residential Water 63c.			
	Arroyo Grande Subbasin		
Year	Estimated Number of Residences <sup>1</sup>	Estimated Water Use (AFY) <sup>2</sup>	
1989	91	73	
1994	93	74	
1999	94	75	
2002	98	78	
2003	101	81	
2007	117	94	
2011	127	102	
2014	136	109	
2019	164	131	

Table 6-7: Rural Residential Water Use.

In addition to the above rural residential water use, there are three commercial operations in the Subbasin that were evaluated separately for water use: Talley Vineyard and Talley Farms in the upper Arroyo Grande Valley, and the Mushroom Farm in Tar Spring Canyon. Square footages of the various buildings were estimated from aerial imagery and multiplied by a nominal water duty factor of 0.06 acre-feet per year per 1,000 square feet, which is considered representative of warehouse, commercial service, and manufacturing (City of San Luis Obispo, 2000). The resulting combined water use for the three commercial operations was 10 acre-feet per year.

# **Agricultural Groundwater Extractions**

Groundwater use for agricultural irrigation has been estimated using the DWR Consumptive Use Program Plus (CUP+; DWR, 2015) which is a crop water use estimator that uses a daily soil moisture balance. CUP+ was developed as part of the 2013 California Water Plan Update to help growers and agencies estimate the net irrigation water needed to produce a crop.

<sup>&</sup>lt;sup>1</sup> outside City limits

<sup>&</sup>lt;sup>2</sup> based on 0.8 AFY per residence

Daily climate data from CIMIS Station #52 (San Luis Obispo) from 1988 to 2020 were used in the CUP+ program, along with estimates for various crop and soil parameters. The climate data is used to determine local reference evapotranspiration (ETo) on a daily basis. Crop coefficients are then estimated for up to four growth stages (initial, rapid, mid-season, late-season) which determine the crop evapotranspiration (ETc) values. Lastly, the CUP+ program uses variables related to the soil and crop type to determine the estimated applied water demand (ETaw), which is equivalent to the net irrigation requirement. Figure 6-10 shows the annual ETaw for various crops during the historical base period, along with ETo and rainfall at CIMIS Station #52.

As noted in Section 6.1, the CIMIS Station at Cal Poly is within DWR reference evapotranspiration Climate Zone 6 (Upland Central Coast; average ETo of 49.7 inches), which is one of 18 climate zones in California based on long-term monthly average reference evapotranspiration (CIMIS, 1999). As shown in the inset in Figure 6-7, most of the Subbasin is within Climate Zone 3 (Coastal Valleys; average ETo of 46.3 inches). Therefore, the reference ETo at Cal Poly would be expected to be greater than in the Subbasin. As previously mentioned in Section 6.1, Nipomo CIMIS Station #202 is within Climate Zone 3, with a historical record going back to 2006 (Figure 6-7 inset). A correlation between the two CIMIS stations shows that the ETo at Station #202 is approximately 83 percent of the ETo at Station #52. Therefore, results of the 1988-2020 soil moisture budget using Station #52 were reduced by 17 percent to better represent the Arroyo Grande Subbasin.

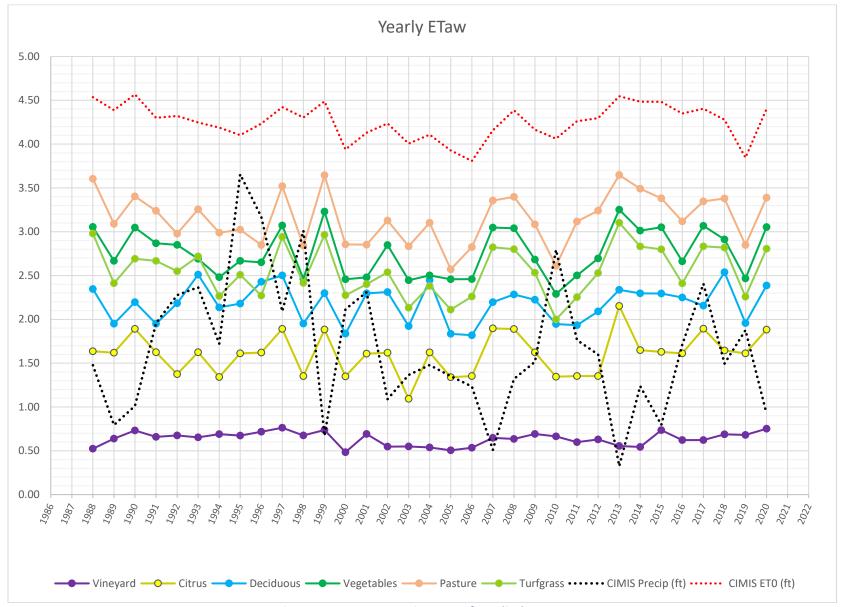


Figure 6-10: Consumptive Use of Applied Water.

Crop types were grouped according to the classification used by County Agricultural Commissioner's Office for crops overlying the Basin. These crop types included citrus, deciduous (non-vineyard), pasture, vegetable, and vineyard. A turf grass classification was added for estimating Urban sector water demand served by groundwater. The CUP+ program provides monthly water demand for each crop type during the hydrologic base period (1988-2020). Low, medium, and high consumptive use of applied irrigation water estimates are presented in Table 6-8. Low and high consumptive use are the respective annual minimum and maximum estimates over the base period, while medium consumptive use is the average. The CUP+ applied water requirement for vegetables was reduced by 40 percent to account for fallow acreage, which is not in production at any given time, based historical aerial image review and discussion with a local grower.

<b>Table 6-8:</b>	Consumptive	Use of	f Applied	Water.
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Table Co.				
Cron Tuno	Acre-feet per acre per year			
Crop Type	Low	Med	High	
Citrus	0.9	1.3	1.8	
Deciduous	1.5	1.8	2.1	
Pasture	2.1	2.6	3.0	
Vegetables*	1.1	1.4	1.6	
Vineyard	0.4	0.5	0.6	
Turfgrass	1.7	2.1	2.6	

<sup>\*60</sup> percent of ETaw to account for fallow fields

As previously discussed in section 6.3.2 (Historical Land Use), the distribution of crop acreage was determined by a review and correlation of DWR and County crop surveys with aerial imagery. Crop acreages were interpolated between the years with data.

Applied water demand volumes were calculated by multiplying the annual acreage for each crop by the average annual applied water demand during each year. The final applied water estimates used for the water budget were adjusted to include efficiency (with system leakage) factors of 80 percent for drip/micro emitter and high-efficiency sprinkler irrigation (citrus, deciduous, vineyard, and turfgrass) and 75 percent for mostly sprinkler with some drip irrigation (pasture and vegetables), based on information from the County Water Master Plan (Carollo, 2012). The estimated groundwater extractions for agricultural water use are shown in the main water budget Table 6-1.

# Subsurface Outflow

Subsurface outflow from Subbasin sediments occurs as underflow through the alluvial deposits of Arroyo Grande Creek. Outflow volumes were estimated using Darcy's Law (see Subsurface Inflow in Section 6.3.4.2). Table 6-9 presents the parameters used for subsurface outflow estimates.

**Table 6-9: Subsurface Outflow Estimates.** 

Cross-sectional Area*	Hydraulic gradient	Hydraulic conductivity	Outflow	
ft²	ft/ft	ft/day	AFY	
170,000	0.01	34	480	

Cross sectional areas for outflow were based on the estimated saturated cross-sectional area of alluvial deposits in the vicinity of where the creek exits the groundwater Subbasin. Hydraulic gradients are the approximate grade of the stream channel, and the hydraulic conductivities are based on pumping tests

(Chapter 5). The outflow estimate is within the range of prior estimates by DWR (2002), but lower than the previous estimate of 2,000 AFY (GSI, 2018), mainly due to a lower hydraulic conductivity based on available pumping tests.

#### 6.3.5 Total Groundwater in Storage

Groundwater is stored within the pore space of Subbasin sediments. The Specific yield is a ratio of the volume of pore water that will drain under the influence of gravity to the total volume of saturated sediments. The specific yield method for estimating groundwater in storage is the product of total saturated Subbasin volume and average specific yield. Calculation of total groundwater in storage for selected years was performed based on the specific yield method.

Estimates of specific yield for Subbasin sediments were obtained based on a review of 19 representative well logs. The lithology for each well log was correlated with specific yield values reported for sediment types in San Luis Obispo County (Johnson, 1967), and were weighted based on the thicknesses of individual sediment types in each log. A summary of the correlations is shown in Table 6-11. Locations of well logs used for the specific yield correlations are shown in the referenced cross-sections from Chapter 4. The average specific yield for the alluvial deposits is estimated at 14.7 percent, compared to 12 percent previously estimated by DWR (2002).

Table 6-10: Specific Yield of Alluvial Deposits.

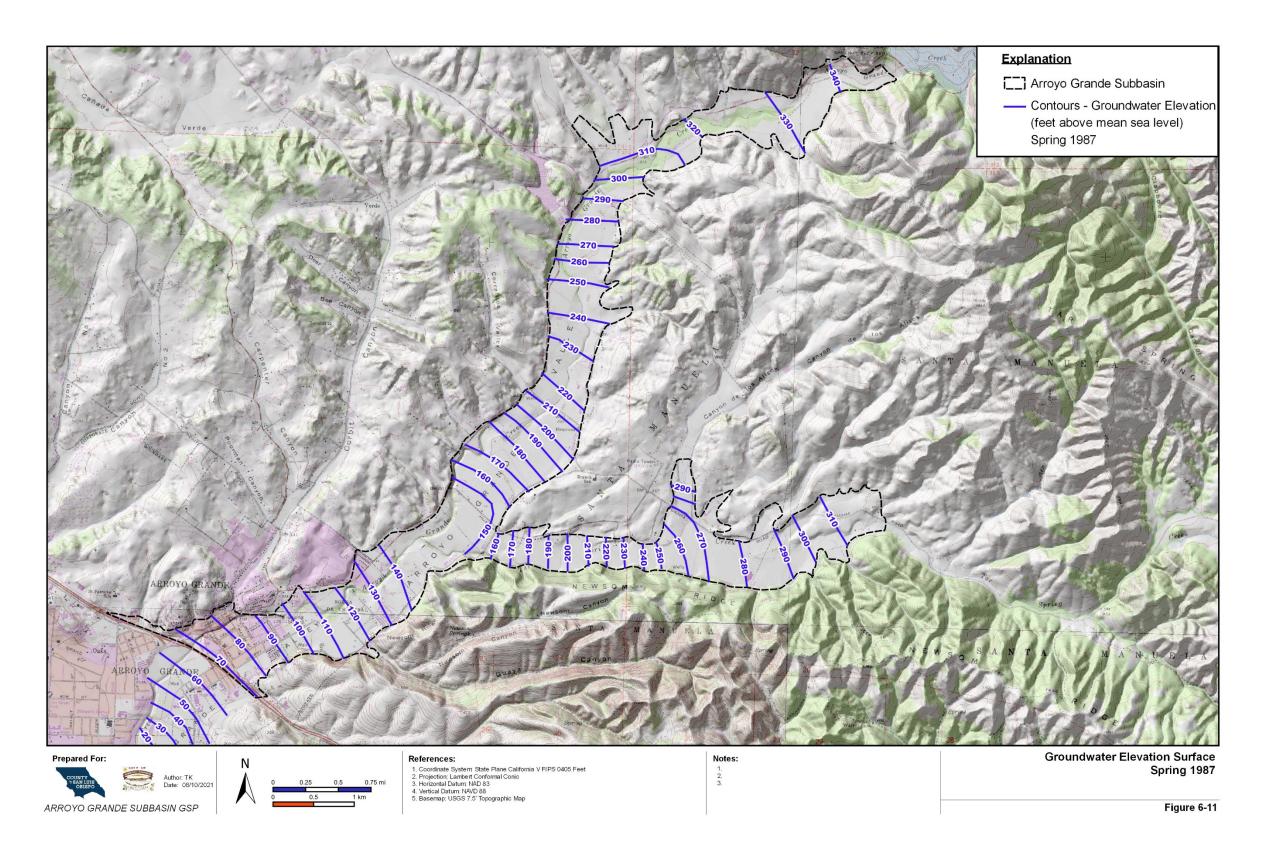
Well ID	Cross- Section	Specific Yield (%)
961610	A'-A"	21.0
1981-003	A'-A"	21.2
E0074069	A'-A"	18.9
WCR2018-06066	A'-A"	17.8
906318	A'-A"	15.8
E0047973	A'-A"	12.6
E0111409	A'-A"	11.8
E0074480	A-A'	15.7
0962373	A-A'	12.6
003929	A-A'	15.4
E0063597	A-A'	11.6
00792659	B-B'	9.7
00335753	B-B'	15.0
00802727	B-B'	16.0
00152206	B-B'	13.9
00738180	B-B'	11.9
00906244	B-B'	14.9
EHS 78-147	A-A' & A'-A"	11.3
EHS 82-51	A-A'	12.9
Average		14.7

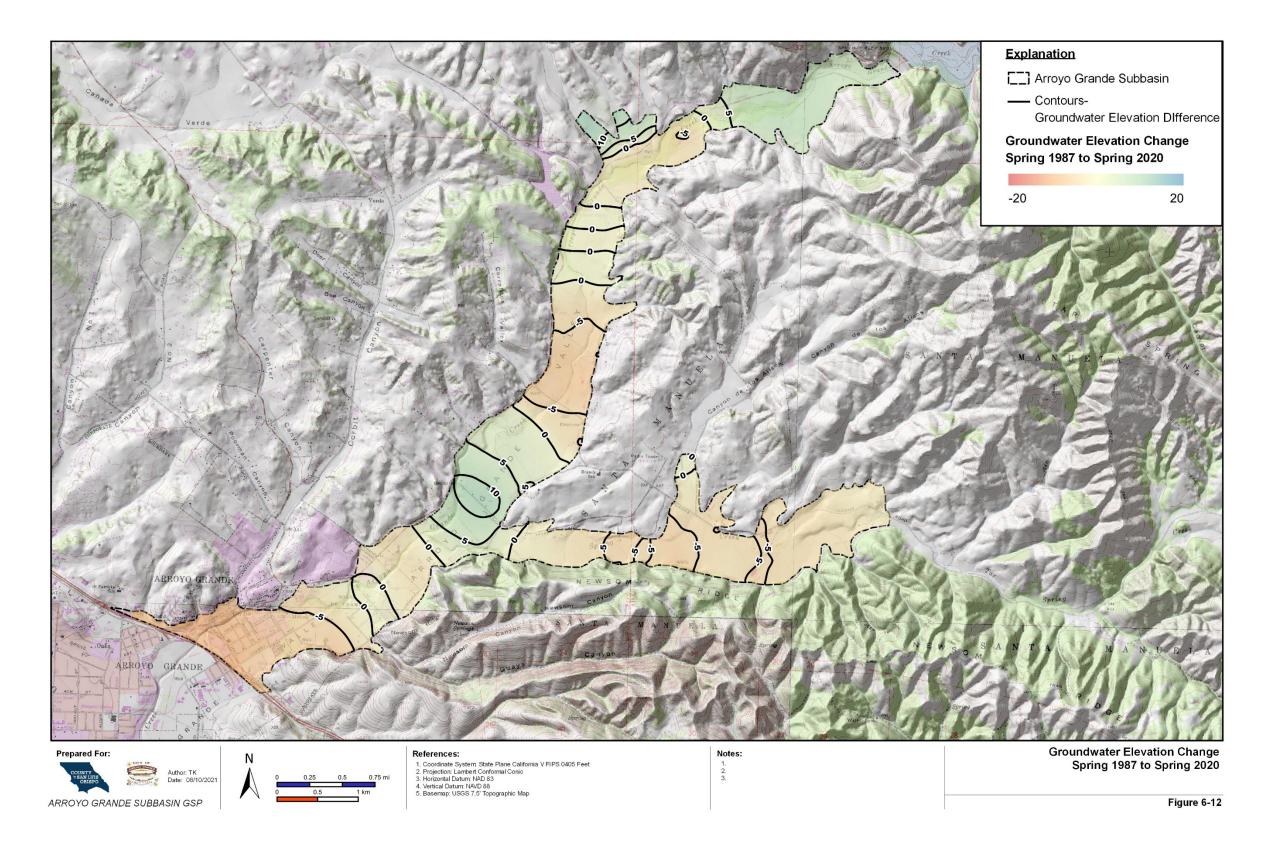
Notes: Cross-sections in Chapter 4 (Figures 4-9, 4-10, 4-11)

Water Budget (§ 354.18)

Groundwater in storage calculations were performed for the Spring conditions of 1987, 1990, 1996, 2002, 2009, 2011, 2015, 2017, and 2020 using the specific yield method. Water level contours for each year were prepared based on available water level data from various sources, including the County water level monitoring program, well logs, and Stakeholder provided information. Water level contour maps for Spring 1996, 2015, and 2020 were shown previously in Chapter 5. Water level contours for Spring 1987 (the start of the historical base period), along with a change in groundwater elevation map from Spring 1987 to Spring 2020 (the end of the historical base period) is shown in Figure 6-11 and Figure 6-12.

The water level contours for storage calculations extend to the Subbasin boundaries. Groundwater levels in the Subbasin in Spring 1987 show a pattern similar to the other contour maps in Chapter 5, including the flattening of the hydraulic gradient in the middle of the Subbasin, where Tar Spring Creek valley enters the Arroyo Grande valley (Figure 6-11). The change in water level elevation map shows relatively minor differences between 1987 and 2020, with fluctuations ranging from five feet of water level decline to 10 feet of water level increase over the base period (Figure 6-12).





The water level contour maps and the base of permeable sediments were processed for volume calculation using Surfer, a grid-based mapping and graphic program. The methodology consisted of gridding and trimming surfaces to the Basin subarea boundaries, followed by volume calculation between surfaces. The gross volumes obtained were then multiplied by the representative specific yield. An example of the methodology showing gridded surfaces for Spring 2020 water levels and the base of permeable sediments is presented in Figure 6-13. Estimated total storage volumes for selected years using the specific yield method are listed in Table 6-11.

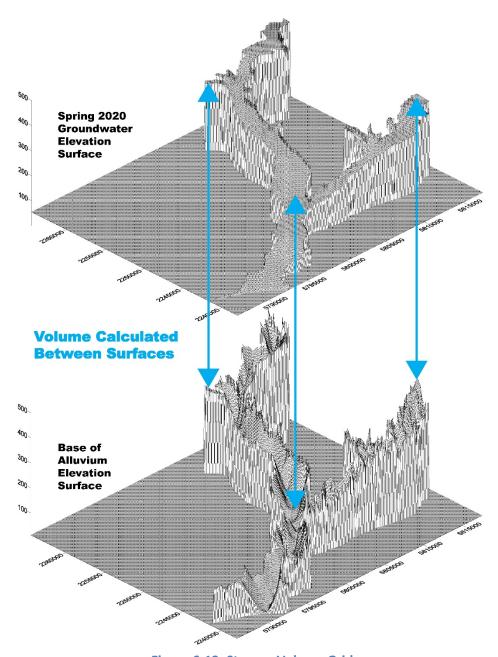


Figure 6-13: Storage Volume Grids.

able 6-11. Spring Groundwater Storage Estima		
Year	Groundwater Storage	
real	Acre-Feet	
1987	13,000	
1990	10,300	
1996	13,700	
2002	13,300	
2009	10,400	
2011	15,200	
2015	10,700	
2017	14,700	
2020	12,800	

**Table 6-11: Spring Groundwater Storage Estimates.** 

The groundwater storage estimates are comparable to previously reported estimate of 14,000 acre-feet total storage capacity for the Arroyo Grande Valley (DWR, 2002). The DWR total storage capacity represented the total volume that could theoretically be held in underground storage. The maximum storage estimated herein by the specific yield method is 15,200 acre-feet (Spring 2011).

## 6.3.6 Change in Storage

Balancing the water budget final step in water budget development. As previously mentioned, the water budget equation is as follows:

INFLOW - OUTFLOW = CHANGE IN STORAGE

The annual change in storage for the surface water budget is assumed to be zero, as surface flow moves quickly through the basin and any differences in storage are minor compared to the total budget. Therefore, the surface water balance equation can be simplified as INFLOW = OUTFLOW, and was used to estimate the stream outflow component of the surface water budget.

For the groundwater budget, groundwater-surface water interaction (as stream flow seepage) was adjusted to approximate the change in storage calculated using the specific yield method discussed above. The difference between the estimated change in storage shown in the water budget and the measured change in storage using the specific yield method is the mass balance error. Change in storage is reported between seasonal high (Spring) conditions per GSP regulations. Change in storage and mass balance error for the groundwater budget is shown in Table 6-12. Figure 6-14 compares storage estimates using the water budget and the specific yield method.

Table 6-12: Change in Storage Comparison – Historical Base Period 1988 – 2020.

Groundwater Budget	Specific Yield Method	Mass Balance Error		e Error
Change in St	orage (acre-feet)	acre-feet	AFY	Percent*
-300	-200	100	3	0

<sup>\*</sup>Percent of total subarea water budget

The difference in change in storage estimates between the water budget and the specific yield method is approximately 100 AFY for the Subbasin over the historical base. The water budget estimates a 300 acrefoot decline in storage, compared to a 200 acrefoot decline in storage using the specific yield method. The difference in change in storage estimates between the water budget and the specific yield method is less than 5 AFY over the historical base period.

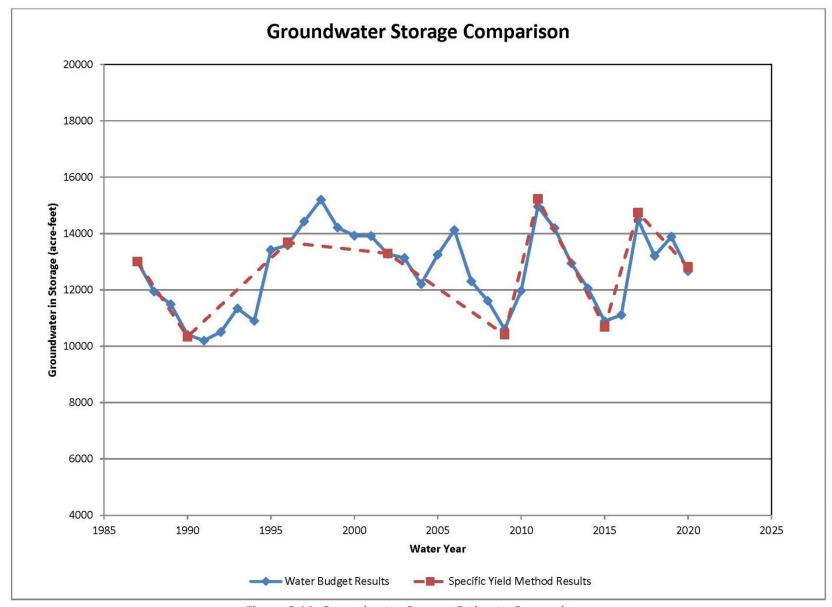


Figure 6-14: Groundwater Storage Estimate Comparison.

## 6.3.7 Preliminary Sustainable Yield Estimate

The sustainable yield is the maximum quantity of water, calculated over a base period representative of long-term conditions in the Subbasin and including any temporary surplus, that can be withdrawn annually from a groundwater supply without causing an undesirable result. Temporary surplus is the amount of water that may be pumped from an aquifer to make room to store future water that would otherwise be unavailable for use. Undesirable results will be defined for six sustainable management criteria in Chapter 7. Examples of potential undesirable results are related to long-term declines in water levels and associated loss of groundwater in storage.

Estimating sustainable yield includes evaluating historical, current, and projected water budget conditions. The analytical water budget method utilized in this analysis evaluates historical and current conditions, and provides a preliminary estimate for the Subbasin sustainable yield. The projected water budget will be evaluated using the Subbasin numerical model presented later in the projected water budget section of the chapter, at which time the minimum thresholds for the sustainable management criteria can be incorporated and the final sustainable yield will be determined. The preliminary sustainability estimate can be used for planning potential projects and management action scenarios for the Subbasin numerical model.

The Arroyo Grande Subbasin has not experienced cumulative and persistent storage declines. The estimated net decline in groundwater storage of less than 10 acre-feet per year over the 33-year historical base period is less than one percent of the annual groundwater budget.

The preliminary sustainable yield of the Arroyo Grande Subbasin is estimated at 2,500 AFY, based on the long-term average recharge of 3,000 AFY minus 500 AFY subsurface outflow (rounded to nearest 100 acrefeet). This preliminary sustainable yield assumes continued operation of Lopez Reservoir in accordance with historical practices. Table 6-13 summarizes the preliminary sustainable yield estimates.

Table 6-13: Preliminary Sustainable Yield Estimate (AFY).

Long-term recharge	3,000
Subsurface outflow	-500
Sustainable Yield	2,500

There are no prior estimates of the Subbasin sustainable yield for comparison. DWR (2002) estimated sustainable yield for portions of the main (downstream) groundwater basin areas. Absent of cumulative and persistent storage declines or other identified undesirable results, the existing level of groundwater basin development may be considered sustainable. It is not a coincidence that Subbasin pumping over the base period for urban and agricultural uses averaged 2,500 AFY, equal to the preliminary sustainable yield.

## 6.3.8 Quantification of Overdraft

Overdraft is the condition of a groundwater basin or subbasin where the amount of water withdrawn by pumping exceeds the amount of water that recharges a basin or subbasin over a period of years, during which the water supply conditions approximate average conditions.

The Arroyo Grande Subbasin is not in overdraft. There have been no significant cumulative and persistent storage declines over the 33-year historical base period. As with the preliminary sustainable yield estimate given above, the absence of overdraft assumes continued operation of Lopez Reservoir in accordance with historical practices.

## **6.4 CURRENT WATER BUDGET**

The current water budget quantifies inflows and outflows for the Subbasin based on the last five years of the historical water budget, from 2016 to 2020. These years provide the most recent population, land use, and hydrologic conditions. Recent Subbasin conditions have been characterized by average rainfall (with wet and dry years), along with a slight increase in urban extractions associated with development projects. There has also been a slight decline in total agricultural acreage and associated groundwater extractions over the last 5 years in the Subbasin, compared to the 33-year base period.

Comparisons of the current water budget to the 1988-2020 historical water budget are shown in Table 6-14, and graphs are shown in Figure 6-15 and Figure 6-16. The average annual surface water budget inflows and outflows are lower for current conditions (averaging 13,090 AFY) compared to the historical base period average of 18,360 AFY. The main reason for the lower total surface water budget for the current condition, despite average rainfall, is a decrease in stream inflow, which was due to the extreme drought that preceded the current condition. Lopez Reservoir was only about 24 percent capacity at the start of the 2016 water year (October 2015), with capacity subsequently doubling by the end of water year 2020. Downstream releases from the reservoir over the current condition were half of the historical average.

The average annual groundwater budget outflows are similar for current conditions (averaging 2,890 AFY) compared to the historical base period average of 2,960 AFY. The groundwater budget inflows, however, are slightly greater for the current condition (3,240 AFY), compared to the historical average of 2,950 AFY. The main reason for the increased inflow is also a response to the preceding drought period. Close to 35 inches of rain fell at Lopez Reservoir in 2017, which replenished soil moisture deficits from the drought and resulted in 3,000 acre-feet of deep percolation across the Subbasin, one of the highest estimated values on record (Table 6-1). Overall groundwater in storage increased an estimated 3,390 acre-feet in 2017. Storage has been generally decreasing since 2017, although there was a net gain over the current condition (2016 through 2020).

**Table 6-14: Current Water Budget.** 

Table 6-14: Current water Budget.			
SURFACE WATER BUDGET	Historical Average (1988-2020)	Current (2016-2020)	
Inflow	AFY		
Precipitation	4,130	4,170	
Groundwater extractions (Urban)	140	180	
Groundwater extractions (Ag)	2,340	2,220	
Stream Inflow at Basin Boundaries	10,910	5,780	
Surface Water Deliveries	450	400	
Local Imported Supplies	390	340	
TOTAL IN	18,360	13,090	
Outflow			
ET of precipitation	2,820	2,910	
ET of Applied Water (Urban)	450	430	
ET of Applied Water (Ag)	1,800	1,720	
Riparian ET	230	230	
Wastewater Export	160	130	
Stream Flow Diversions	450	400	
Infiltration of Precipitation	970	1,040	
Infiltration of Applied Water			
(Urban)	80	90	
Infiltration of Applied Water (ag)	540	500	
GW-SW interaction (net)	1,200	1,450	
Stream outflow at basin boundary	9,680	4,190	
TOTAL OUT	18,360	13,090	
GROUNDWATER BUDGET	Historical Average (1988-2020)	Current (2016-2020)	
Inflow	AFY		
Infiltration of precipitation	970	1,040	
Urban water return flow	80	90	
Agricultural return flow	540	500	
GW-SW interaction (net)	1,200	1,450	
Subsurface from bedrock	170	170	
TOTAL IN	2,950	3,240	
Outflow			
Groundwater extractions (Urban)	140	180	
Groundwater extractions (Ag)	2,340	2,220	
Subsurface outflow	480	480	
TOTAL OUT	2,960	2,890	

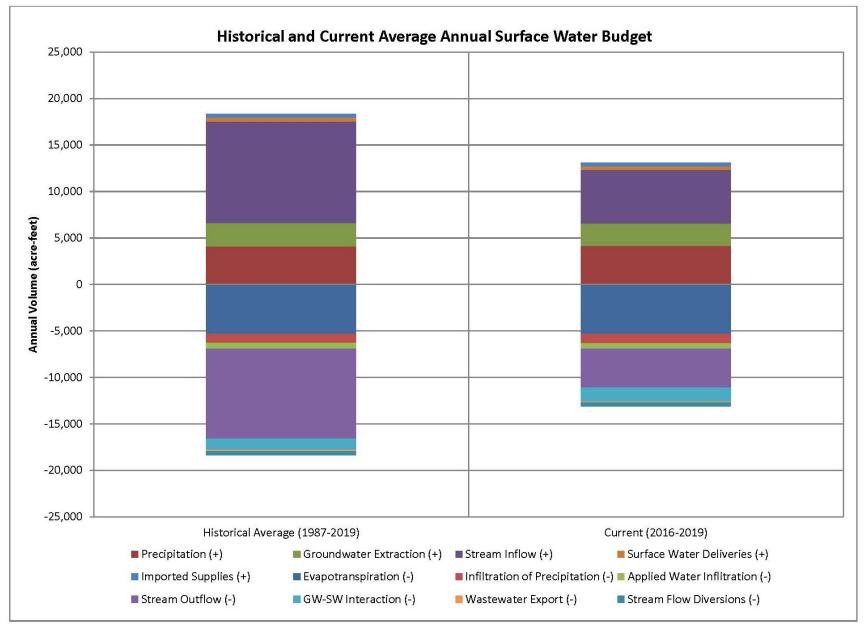


Figure 6-85: Historical and Current Average Annual Surface Water Budget – Arroyo Grande Subbasin

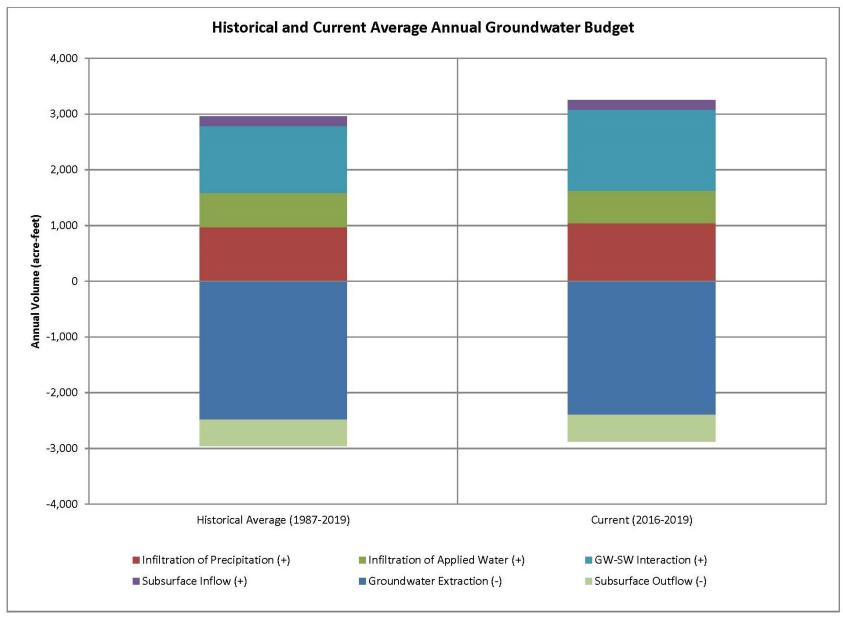


Figure 6-96: Historical and Current Average Annual Groundwater Budget – Arroyo Grande Subbasin.

The graphs in Figures 6-16 and 6-17 provide a visual comparison of the magnitude of components of inflow and outflow listed in Table 6-14. The surface water budget (Figure 6-16) is balanced (total inflow equals total outflow), while the groundwater budget (Figure 6-17) depicts a relatively balanced historical period with a net increase of inflow compared to outflow for current conditions

# 6.5 FUTURE WATER BUDGET

- 6.5.1 Assumptions
- **6.5.2 Inflows**
- 6.5.3 Outflows
- 6.5.4 Change In Storage

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